

22 March 2024

Venetin Aghostin Senior Development Planner Fairfield City Council

ADDENDUM TRAFFIC & PARKING STATEMENT IN REPLY TO COUNCIL'S RFI LETTER PROPOSED RESIDENTIAL FLAT BUILDING & TOWNHOUSES DEVELOPMENT 400-404 CABRAMATTA ROAD WEST, CABRAMATTA, 2 ORANGE GROVE ROAD, CABRAMATTA, AND 6 LINKS AVENUE, CABRAMATTA (DA NO. 260.1/2023)

We refer to Council's RFI letter dated 21 December 2023 in relation to DA No. 260.1/2023 for the proposed residential flat building (RFB) and townhouses development at 400-404 Cabramatta Road West, Cabramatta, 2 Orange Grove Road, Cabramatta, and 6 Links Avenue, Cabramatta, and provide the following additional assessment and information for Council's consideration.

This traffic Statement has been prepared for Tcon Constructions and is to be read in conjunction with the amended architectural plans for the residential flat building prepared by Aleksandar Projects and for the townhouses prepared by Designiche Building Designers (reduced copy of both sets of amended plans – Revision C - attached in **Appendix 'A' and 'B'** of this statement).

1. Vehicular Access and Traffic Impacts

b) The proposed development layout has been amended to provide 'STOP signage at the egress point from the site into Links Avenue, giving priority to traffic travelling along Links Avenue, and in accordance with Clauses 1.5.1 viii. and xi. of SSDCP. It is recommended to extend the existing 'No Stopping' signage located along both sides of Links Avenue, between the bend on Links Avenue and the intersection with Orange Grove Road. Further, there is the proposed extension of the existing road BB line marking to be 30 meters in length within Links Avenue at its approach to its signalised intersection with Orange Grove Road.

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This will alleviate potential traffic impacts on residents on Links Avenue, preventing congestion from vehicles entering and exiting the street as well as the subject site driveway. Refer to **Appendix 'J'** for the attached aerial map of Links Avenue with the proposed signage and BB line marking extensions.

Traffic calming measures have been provided within the internal roads of the development layout as shown in the proposed on-site signage plan in **Appendix 'K'**, including speed humps and dedicated pedestrian walkways, with associated '10 km/h Shared Zone' signage, in order to reduce speed environment, raise awareness of the presence of pedestrians and children and increase safety for all users of the carpark.

- c) Hemanote Consultants have consulted with TfNSW on Council's concerns regarding the proposed development having only one vehicular access from Links Avenue. As shown in **Appendix 'F'** of this statement, TfNSW has advised that vehicular conflict points along arterial roads are to be limited in order to maintain network safety and efficiency. Given that Orange Grove Road and Cabramatta Road West are major arterial roads, carrying high volumes of traffic, vehicular access is not to be provided through these roads. TfNSW has specifically stated, "any future development of the site to not have vehicular access on the classified road frontage, with all vehicular access to be provided *via Links Road*". This is also in line with Clause 1.5.1 x. of SSDCP, TfNSW current publication of the *Guide to Traffic Generating Developments* and Clause 2.119 of *State Environmental Planning Policy (Infrastructure) 2021*.
- d) It is recommended that vehicular access to the residential flat building towards the north of the site be restricted to the eastern internal road of the development. This is to limit access to the number of vehicles passing through the western internal road where a majority of the residential townhouses are proposed and reduce the adverse impacts on privacy. This is to be accompanied by the proposed traffic control measures, including speed humps to reduce vehicle speed, as well as signage.

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This includes signage to be provided at the end of the main access road into the site, showing 'Apartment Building All Traffic' with an associated right turn arrow. In addition, 'left-turn only' signage out of the basement car park for the RFB and left-turn arrows are to be provided. Refer to **Appendix 'J'** for the proposed signage plan.

- e) Line-marked dedicated pedestrian walkways have been provided within the internal roadway, to provide a defined pedestrian path across the site. The on-site parking area is also signposted with a speed limit of '10 km/h Shared Zone' to reduce the speed environment, raise awareness of the presence of pedestrians and children and increase safety for all users of the carpark.
- f) 'No Parking' signage is to be posted along either side of all internal circulation roadways as shown in **Appendix** 'K', in order to prevent residents and visitors from parking in front of the residential townhouses.

4. Inconsistencies with Site Specific DCP (SSDCP) Controls

- a) a. It is recommended to extend the existing 'No Stopping' signage located along the northern and southern sides of Links Avenue, between the bend on Links Avenue and the intersection with Orange Grove Road. This will mitigate potential traffic impacts on residents on Links Avenue, preventing congestion from vehicles entering and exiting the street as well as the site driveway. This is as demonstrated in **Appendix 'J'** of this statement.
- a) j. The proposed internal carriageways of the site have a minimum width of 6 meters between kerbs, as per Clause 1.5.1 iv. of SSDCP, which is adequate for vehicles to access the proposed garages and provide two-way manoeuvring around the site. Additionally, 'No Parking' signage is to be posted along either side of all internal circulation roadways, as shown in **Appendix 'K'**, to prevent residents and visitors from parking in front of the residential townhouses.

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b) The on-site basement car parking areas have been amended to provide a total of 22 visitor car parking spaces in compliance with Chapter 12, Section 12.1, Table 1, of Fairfield CityWide Development Control Plan 2013, in addition to a separate 3 metres wide car wash bay.

h) The proposed at-grade car parking layout has been amended to remove the mentioned car parking spaces, providing a landscaped and common space area at the end of Block D, as additional landscape next to U.1 townhouse.

k) A Pedestrian Access and Mobility Plan (PAMP) has been prepared by Hemanote Consultants as per Council's request and in line with Clause 1.5.1 ix. of SSDCP, in order to provide a framework for developing infrastructure strategies and pedestrian routes to improve the accessibility, safety, convenience and connectivity of all pedestrians within the local area. The PAMP is provided separately from this statement.

5. Inconsistencies with the ADG

Vehicle Access Points

a) The proposed development plans have been amended to provide on-site signage between all conflict vehicle points, specifically between basement and at-grade level vehicular and pedestrian access. 'STOP' signage has been installed at the basement entry/exit point, giving priority to traffic within the internal circulation roadways, improving traffic flow and reducing vehicle conflict. Additional speed humps, dedicated pedestrian walkways, and '10 km/h Shared Zone' signage have been provided as calming measures to slow down vehicles and raise awareness of the presence of both pedestrians and vehicles on-site. This is as demonstrated in **Appendix 'K'** of this statement.

b) Between the intersection of the basement access, loading bay and circulation roadway, the installation of 'STOP' and 'Watch for Reversing Truck' signage has been provided as a measure to mitigate conflict between vehicular manoeuvring within this area of the site.

In addition, to further improve traffic flow within the site, it is recommended that all traffic access to the RFB be restricted to the eastern side of the circulation roadway and signage be provided at the end of the main access road into the site, Herowing Opposition Physical Republic All ABN 94 606 345 117

Traffic', with an associated right turn arrow.

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In addition, 'left-turn only' signage out of the basement car park for the RFB and left turn arrows are to be provided. This is to limit access to the number of vehicles passing through the western circulation roadway where pedestrian walkways are proposed, therefore improving pedestrian safety.

d) Pedestrian and vehicle access has been separated to the site, where vehicular access is directly through Links Avenue and into the proposed site driveway, whereas pedestrian access is through separate access walkways leading from Orange Grove Road / Cumberland Highway and Cabramatta Road West, as shown in **Appendix 'A' and 'B'**. Direct vehicular access from Links Avenue to the proposed RFB is through the eastern internal road as previously mentioned. Pedestrian access is through the mentioned walkways from Orange Grove Road / Cumberland Highway and Cabramatta Road West, which is safer for pedestrians as they do not have to cross through the main driveway access and within the internal roadway to get to the RFB.

Car Parking and Basement

a) Three charging stations for electric vehicles have been provided within basement 1 of the RFB.

6. Transport for NSW (TfNSW)

Traffic Volume Surveys

Traffic volume surveys were undertaken by *Geocounts Data Supply* on behalf of Hemanote Consultants at the following intersections:

- Cumberland Highway / Cabramatta Road West / Orange Grove Road
- Orange Grove Road / Links Avenue / Golf Club Access
- Orange Grove Road / Viscount Place

near the subject site on Thursday 22 February 2024, during morning period (6.30am to 9.30am) and afternoon period (2.30pm to 6.30pm), considering traffic peak periods. Refer to **Appendix 'G'** for the attached traffic survey results.

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Estimated Future Traffic Generation

An indication of the potential traffic generation of the proposed development is provided by the RMS *Guide to Traffic Generating Development - 2002*.

The Guide specifies the following traffic generation rates for **medium-density** residential developments:

Larger units and townhouses (three or more bedrooms):

Weekday peak hour vehicle trips = 0.5-0.65 per dwelling.

Therefore, the proposed townhouses with fifty-three (53) residential apartments (32 \times 3-bed units and 21 \times 4-bed units) have an estimated traffic generation as follows:

•27 to 35 peak hour vehicle trips (In and Out trips).

Further, as requested by TfNSW, given the lack of public transport opportunities in close proximity to the subject site, a re-estimated traffic generation rate for **high-density** residential developments has been provided as follows:

- AM peak (1 hour) vehicle trips per unit = 0.26.
- PM peak (1 hour) vehicle trips per unit = 0.29.

Therefore, the proposed RFB with eighty-five (85) residential units has an estimated traffic generation of:

- 22 AM peak <u>hour vehicle</u> trips (In and Out trips).
- 25 PM peak hour vehicle trips (In and Out trips).

The total estimated peak hour traffic generation from the proposed development is:

- 57 AM peak <u>hour vehicle</u> trips (20% Inbound and 80% Outbound trips for typical residential development)
- 60 PM peak <u>hour vehicle</u> trips (80% Inbound and 20% Outbound trips for typical residential development)

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Intersection Performance (Pre & Post-development)

Average Vehicle Delay (AVD) and Level of Service (LOS) – The AVD and LOS provides a measure of the operational performance of an intersection, as indicated in Table 4.2 of the Roads & Maritime Services "Guide to Traffic Generating Developments - 2002" (shown below).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
А	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays	At capacity, requires other control mode
		Roundabouts require other control mode	

Table 4.2: Level of Service Criteria for intersections (RMS Guide)

A **pre-development** SIDRA intersection performance analysis was undertaken for the existing intersections of Cumberland Highway / Cabramatta Road West / Orange Grove Road, Orange Grove Road / Links Avenue / Golf Club Access and Orange Grove Road / Viscount Place, in the vicinity of the subject site.

The SIDRA base model has incorporated the information provided in the Traffic Control Signal plans and the signal phase times obtained from TfNSW. Subsequently, the SIDRA base model was calibrated against the observed field back-of-queue length at the start of the green signals to establish a calibrated base.

It is important to note that data from the observed queue length at each approach leg of the intersections during the peak AM & PM hours were utilised to calculate the actual average and 95th percentile back-of-queue length. This data was then compared against the SIDRA base model using the estimated values in the Queue Analysis Report, which includes the Basic Saturation Flow parameter.

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Please refer to **Appendix 'H'**, which summarises the average and 95th percentile back-of-thequeue lengths at the start of the green light obtained from the field and the SIDRA basic model. Additionally, it presents the new values based on the calibrated model, along with the parameters used for the calibration.

Refer to Figure 1 on the following page, showing the intersections network layout controlled by traffic signalling systems. Cabramatta Road West, Cumberland Highway, Orange Grove Road and Viscount Place have divided carriageways, with three through traffic lanes in each direction for Cabramatta Road West, Cumberland Highway and Orange Grove Road, and two through traffic lanes for Viscount Place in each direction. Links Avenue has an undivided carriageway with one through-traffic lane in each direction.

A post-development SIDRA intersection performance modelling analysis was then undertaken for the subject intersections, and it was modelled as the proposed network layout as shown in Figure 1 on the following page.

Refer to the summary of the results of the SIDRA intersection performance analysis (undertaken for pre & post-development) attached in **Appendix 'I'** of this statement.

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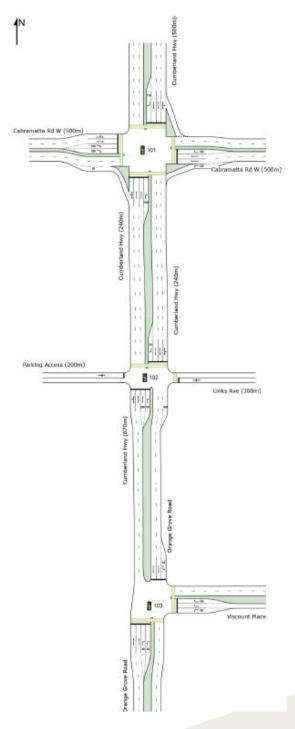


Figure 1: Intersection Network Layout

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The following <u>assumptions</u> have been considered and adopted in the SIDRA Network Intersection modelling for the post-development conditions considering that the main access to and from the subject site is through Links Avenue:

- Morning and afternoon peak traffic generation was adopted as per the future traffic generation in the previous section of the statement.
- Traffic generated by the development was split into inbound and outbound traffic for AM
 PM peak hours as outlined in future traffic generation of the statement.
- Pre-development network analysis is modelled for the base year (2024) in surrounding traffic.
- Post-development network analysis is modelled for when the residential development is in operation.

These assumptions will result in the development trip distribution shown in Figures 2 and 3 for relevant traffic movement and modelled intersections.

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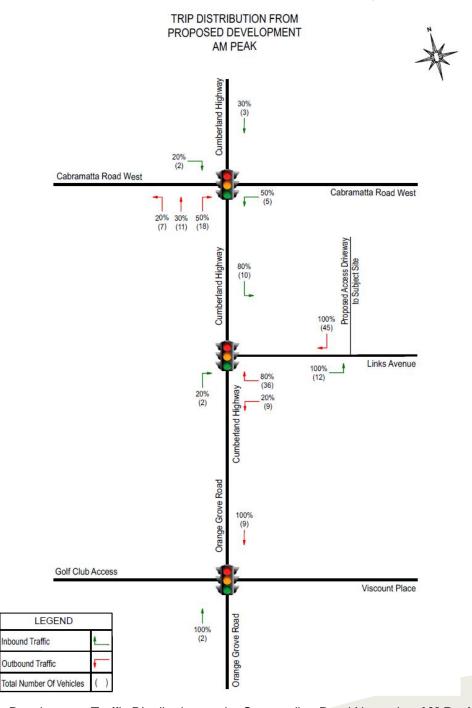


Figure 2: Development Traffic Distribution on the Surrounding Road Network - AM Peak

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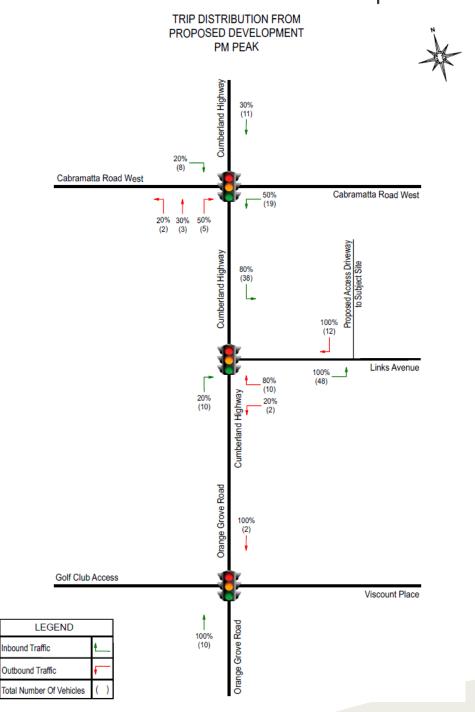


Figure 3: Development Traffic Distribution on the Surrounding Road Network - PM Peak

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The outcome of the SIDRA modelling

A summary of the results of the SIDRA intersection performance analysis has been provided in Tables 1 to 2 below, as well as the SIDRA Movement Summary Tables attached in **Appendix 'I'** of this statement.

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	Approach Leg	ch Traffic Movement		Pre De	velopmen	it	Post Development				
Intersection			LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	
	South: Orange Grove Road	Left Turn into Cabramatta Road West	А	7.3	0.723	31.3	А	7.2	0.731	31.9	
	South: Orange Grove Road	Through	D	44.7	0.723	31.3	D	45.1	0.731	31.9	
	South: Orange Grove Road	Right Turn into Cabramatta Road West	F	174.9	1.088	21.7	F	259.3	1.190	28.7	
	East: Cabramatta Road West	Left Turn into Orange Grove Road	А	14.3	0.222	5.9	А	14.4	0.228	6	
	East: Cabramatta Road West	Through	D	49.4	0.565	16	D	49.4	0.565	16	
Cumberland Highway / Cabramatta	East: Cabramatta Road West	Right Turn into Cumberland Highway	F	75	0.605	6.5	F	75	0.605	6.5	
Road West / Orange Grove Road	North: Cumberland Highway	Left Turn into Cabramatta Road West	В	24.7	0.599	22.4	В	24.8	0.6	22.5	
	North: Cumberland Highway	Through	С	38.5	0.599	23.1	С	38.6	0.6	23.2	
	North: Cumberland Highway	Right Turn into Cabramatta Road West	F	77.7	0.763	9.8	F	77.7	0.763	9.8	
	West: Cabramatta Road West	Left Turn into Cumberland Highway	F	95.4	0.984	46.3	F	95.5	0.984	46.3	
	West: Cabramatta Road West	Through	F	118.3	0.984	46.3	F	118.4	0.984	46.3	
	West: Cabramatta Road West	Right Turn into Orange Grove Road	F	90	0.878	10.3	F	90.7	0.885	10.4	

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				Pre De	velopmen	t	Post Development				
Intersection	Approach Leg	Traffic Movement	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	
	South: Orange Grove Road	Left Turn into Golf Club Access	А	11.7	0.304	8.5	А	12.3	0.312	9	
	South: Orange Grove Road	Through	А	5.3	0.304	8.6	А	5.9	0.312	9.2	
	South: Orange Grove Road	Right Turn into Links Avenue	F	77.7	0.014	0.1	F	79.1	0.042	0.2	
	East: Links Avenue	Left Turn into Orange Grove Road	E	64.9	0.346	3.2	E	65.4	0.520	6.5	
	East: Links Avenue	Through	Е	69.4	0.346	3.2	Е	69.4	0.520	6.5	
Orange Grove Road /	East: Links Avenue	Right Turn into Orange Grove Road	F	73.9	0.346	3.2	F	74.5	0.520	6.5	
Links Avenue / Golf Club Access	North: Orange Grove Road	Left Turn into Links Avenue	А	11.3	0.107	2.5	А	11.2	0.111	2.8	
	North: Orange Grove Road	Through	А	4.5	0.107	2.6	А	5	0.111	2.8	
	North: Orange Grove Road	Right Turn into Golf Club Access	F	77.5	0.013	0.1	F	78.6	0.013	0.1	
	West: Golf Club Access	Left Turn into Orange Grove Road	E	61.8	0.023	0.2	F	144.1	0.0	0.3	
	West: Golf Club Access	Through	Е	61.1	0.023	0.2	F	161.3	0.0	0.3	
	West: Golf Club Access	Right Turn into Orange Grove Road	E	72.9	0.023	0.2	F	170.6	0.0	0.3	

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Intersection	7 .pp. 0			Pre De	velopmen	ıt	Post Development				
		Traffic Movement	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	
	South: Orange Grove Road	Through	А	5.8	0.786	32	А	5.8	0.787	32.1	
	South: Orange Grove Road	Right Turn into Viscount Place	F	70.7	0.835	5	F	70.7	0.835	5	
Orange Grove Road /	East: Viscount Place	Left Turn into Orange Grove Road	D	48.6	0.245	3.6	D	48.6	0.245	3.6	
Viscount Place	East: Viscount Place	Right Turn into Orange Grove Road	E	64.5	0.644	4.2	E	64.5	0.644	4.2	
	North: Orange Grove Road	Left Turn into Viscount Place	А	11.9	0.185	2.9	А	11.9	0.185	2.9	
	North: Orange Grove Road	Through	В	17.7	0.864	47.4	В	18.3	0.868	48.4	

Table 1: Existing Intersection Performance - AM Peak (Base Year 2024)

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				Pre De	velopmen	ıt	Post Development				
Intersection	Approach Leg	Traffic Movement	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	
	South: Orange Grove Road	Left Turn into Cabramatta Road West	D	49.9	0.992	54.2	D	51.4	0.995	54.2	
	South: Orange Grove Road	Through	F	112.5	0.992	54.2	F	114.2	0.992	54.2	
	South: Orange Grove Road	Right Turn into Cabramatta Road West	F	96.4	0.962	26.9	F	102.6	0.978	28.3	
	East: Cabramatta Road West	Left Turn into Orange Grove Road	A	12.6	0.209	5.5	А	12.8	0.227	6.1	
	East: Cabramatta Road West	Through	F	81.4	0.957	39.2	F	81.4	0.957	39.2	
Cumberland Highway / Cabramatta	East: Cabramatta Road West	Right Turn into Cumberland Highway	F	74.7	0.584	6.2	F	74.7	0.584	6.2	
Road West / Orange Grove Road	North: Cumberland Highway	Left Turn into Cabramatta Road West	В	25.8	0.67	22.1	В	26.6	0.663	22.4	
	North: Cumberland Highway	Through	D	48.2	0.657	22.5	D	48.4	0.663	22.7	
	North: Cumberland Highway	Right Turn into Cabramatta Road West	F	80.6	0.898	22.4	F	80.6	0.898	22.4	
	West: Cabramatta Road West	Left Turn into Cumberland Highway	С	3.8	0.825	27.9	С	3.8	0.826	27.9	
	West: Cabramatta Road West	Through	E	60.9	0.825	27.9	Е	60.9	0.826	27.9	
	West: Cabramatta Road West	Right Turn into Orange Grove Road	F	77	0.660	7.1	F	77.5	0.687	7.5	

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				Pre De	velopmen	ıt	Post Development				
Intersection	Approach Leg	Traffic Movement	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	
	South: Orange Grove Road	Left Turn into Golf Club Access	В	17.5	0.768	21.4	В	16.6	0.768	20.9	
	South: Orange Grove Road	Through	А	11.1	0.768	22.4	А	10.2	0.768	21.4	
	South: Orange Grove Road	Right Turn into Links Avenue	F	79.6	0.110	0.6	F	79.2	0.243	1.3	
	East: Links Avenue	Left Turn into Orange Grove Road	E	63	0.356	2.4	Е	66.4	0.574	3.4	
	East: Links Avenue	Through	E	67.4	0.356	2.4	F	70.9	0.574	3.4	
Orange Grove Road /	East: Links Avenue	Right Turn into Orange Grove Road	F	74.1	0.356	2.4	F	78.0	0.574	3.4	
Links Avenue / Golf Club Access	North: Orange Grove Road	Left Turn into Links Avenue	А	11.6	0.112	2.9	А	10.6	0.120	2.9	
	North: Orange Grove Road	Through	А	5.1	0.112	2.9	А	4.6	0.120	3	
	North: Orange Grove Road	Right Turn into Golf Club Access	F	79.4	0.106	0.6	F	78.3	0.104	0.6	
	West: Golf Club Access	Left Turn into Orange Grove Road	E	61.6	0.225	1.9	Е	62.9	0.249	2	
	West: Golf Club Access	Through	Е	65.8	0.225	1.9	Е	67.2	0.249	2	
	West: Golf Club Access	Right Turn into Orange Grove Road	F	71.3	0.225	1.9	F	72.8	0.249	2	

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	Approach Leg			Pre De	velopmen	nt	Post Development			
Intersection		Traffic Movement	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)	LOS	Avg Delay (sec)	DOS (Veh/C)	Back of Queue (Veh)
	South: Orange Grove Road	Through	А	8	0.295	4.1	А	8	0.3	4.2
	South: Orange Grove Road	Right Turn into Viscount Place	С	31	0.8	7.9	С	31	0.8	7.9
Orange Grove Road /	East: Viscount Place	Left Turn into Orange Grove Road	А	14.4	0.707	13.4	А	14.4	0.707	13.4
Viscount Place	East: Viscount Place	Right Turn into Orange Grove Road	С	29.7	0.813	10.4	С	29.7	0.813	10.4
	North: Orange Grove Road	Left Turn into Viscount Place	А	12.5	0.435	6.4	А	12.5	0.435	6.4
	North: Orange Grove Road	Through	В	25	0.775	7.1	В	25.1	0.778	7.2

Table 2: Existing Intersection Performance - PM Peak (Base Year 2024)

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It can be concluded from the results of the SIDRA modelling that:

AM Base Year

- The current overall operational performance (pre-development) at the cross-intersection of Orange Grove Road and Links Avenue is operating at a Level of Service (LOS) 'B' during AM peak periods.
- The proposed residential development will generally not alter the current Level of Service (LOS) at the subject intersection, with the exception of the traffic movements at the west approach leg (Golf Club access road), where the average delay will increase. It should be noted that the traffic count survey indicated that there is no through traffic at this leg, and the SIDRA model has to be included as it is permitted. However, if the through traffic movement is restricted and the traffic movements are limited to right and left turns only into Orange Grove Road, then the average delay will revert back to the pre-development condition.
- The vehicle queue length on Links Avenue at the existing intersection will increase from 23.6m to 46.4m (an increase of 3 vehicles). This is acceptable, and the post-development queue length will be well within the distance between the proposed access driveway and the intersection and will not impact vehicular access for the subject site.
- Furthermore, the proposed residential development will not alter the LOS at Cumberland Highway / Cabramatta Road West / Orange Grove Road and Orange Grove Road / Viscount Place intersections.

PM Base Year

- The current overall operational performance (pre-development) at the cross-intersection of Orange Grove Road and Links Avenue is operating at a Level of Service (LOS) 'A' durning PM peak periods. The proposed residential development will generally not alter the current LOS at the subject intersection.
- The vehicle queue length on Links Avenue at the existing intersection will increase from 17.3m to 24.3m (an increase of 1 vehicle). This is acceptable and the post development queue length will be well within the distance between the proposed access driveway and the intersection.

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 Furthermore, the proposed residential development will not alter the LOS at Cumberland Highway / Cabramatta Road West / Orange Grove Road and Orange Grove Road / Viscount Place intersections.

Therefore, the estimated traffic generation from the proposed development is of low impact on existing flows on Orange Grove Road, Links Avenue and surrounding streets and will not have adverse impacts on the current operational performance of the subject existing intersections, which will generally continue to operate at the same level of service.

The additional traffic generated by the proposed development can be readily accommodated within the existing road layout, without adverse impacts on the amenity of the area

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6. d) It is not recommended to restrict vehicular movements to 'left-in, left-out' at the site access driveway, given that Links Avenue has a Cul-de-sac dead end to its south, and in general, a majority of vehicles exiting the site, are likely to turn right into Links Avenue and head towards Orange Grove Road. It is not suggested for vehicles to left-turn into Links Avenue out of the site, as vehicles will be forced to undertake a 3-point turn in order to travel back towards Orange Grove Road, causing adverse traffic impacts on Links Avenue.

7. Transport and Traffic Engineering Branch Comments

- c) Vehicular swept paths have been undertaken at the site access driveway demonstrating a HRV entering and exiting the site in a forward direction, as well as manoeuvring throughout the internal circulation roadway. Refer to **Appendix 'E'** for heavy vehicle swept paths. In addition, swept paths have been carried out showing a B99 passing a B85 passenger vehicle simultaneously at the access driveway as well as at intersection areas. Refer to the updated vehicle swept path plans attached in **Appendix 'C'**.
- d) A Loading Management Plan (LMP) has been prepared by Hemanote Consultants to address the frequency and type of vehicles expected to service the site and utilise the on-site loading bay. The LMP is provided separately from this statement.

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Given the additional traffic assessment and information provided in this statement, it is considered that the proposed amended on-site vehicular access, traffic and parking layout for the subject residential development is satisfactory and compliant with the requirements of AS2890.1:2004, AS2890.2:2018, and AS2890.6:2022, addressing both Council's and TfNSW's concerns.

If you require any further information, please do not hesitate to contact us.

Yours sincerely

Ramy Selim

Senior Traffic Engineer

Hemanote Consultants Pty Ltd

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Appendix A – Amended Development Architectrual Plans (Townhouses)

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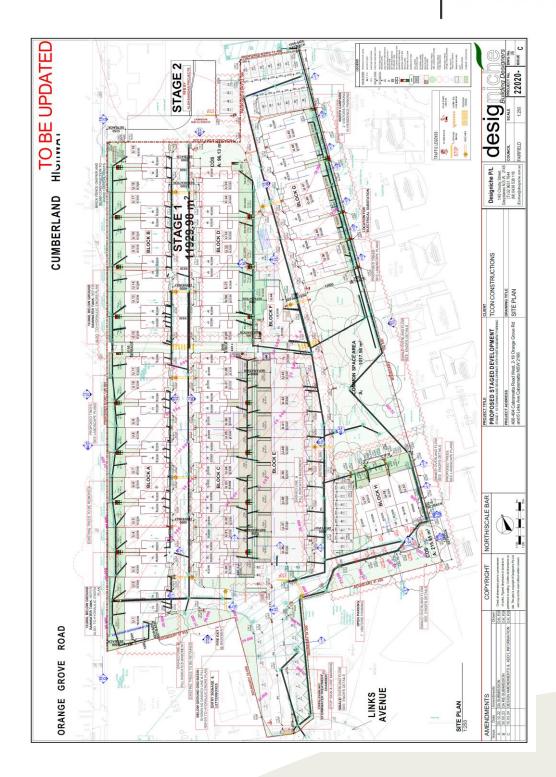
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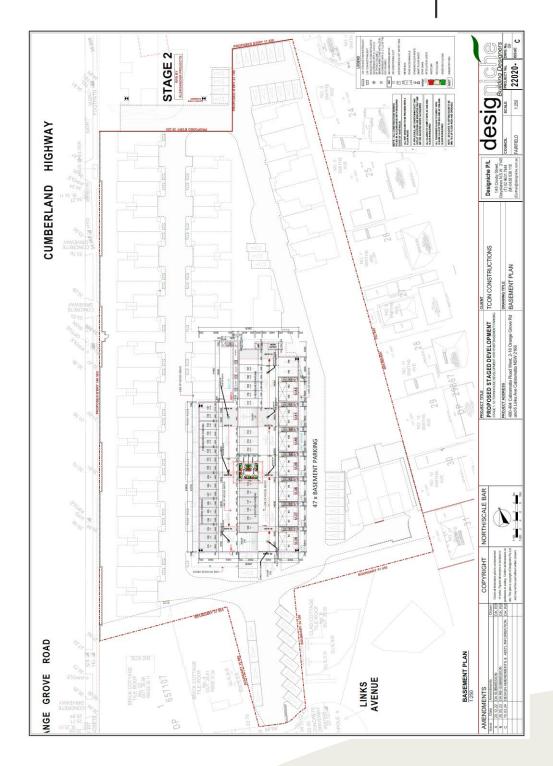
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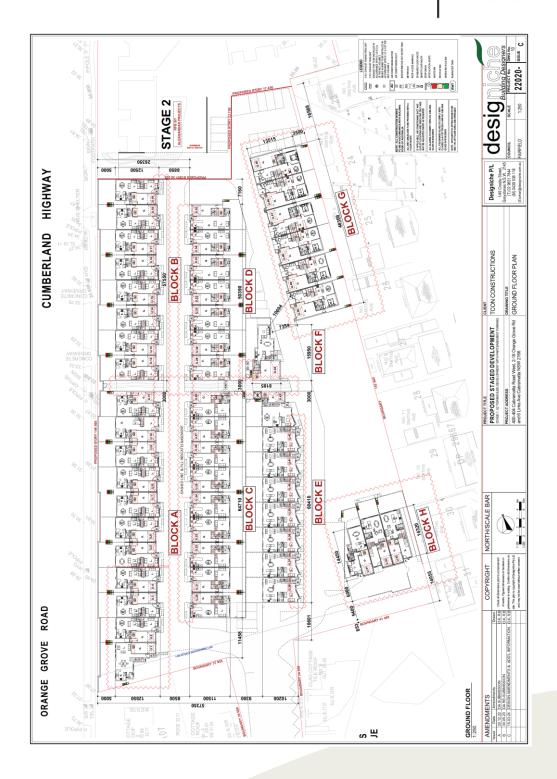
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<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

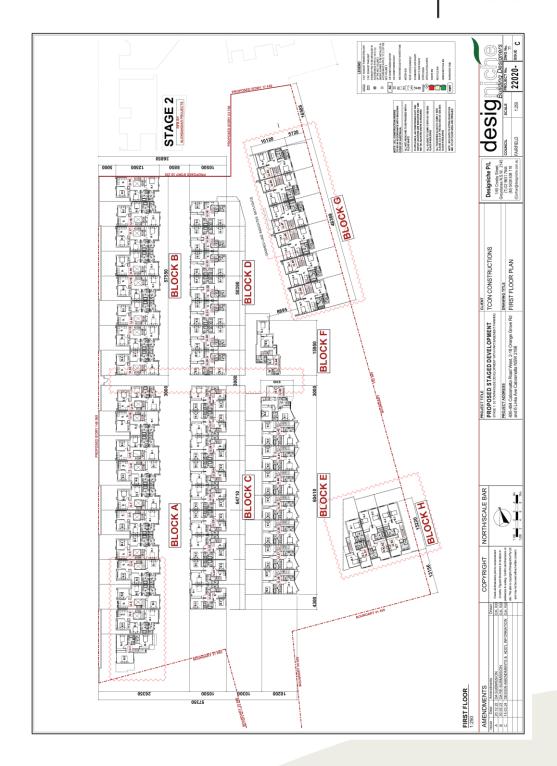
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<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

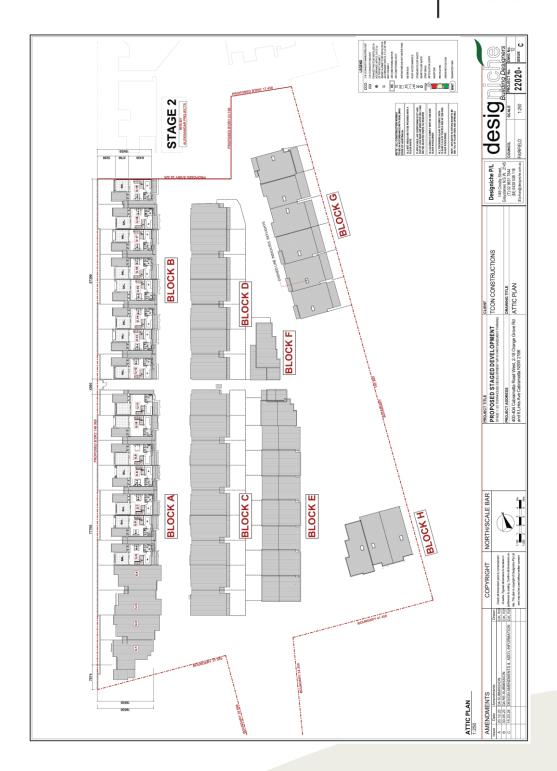
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Appendix B – Amended Development Architectrual Plans (RFB)

<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

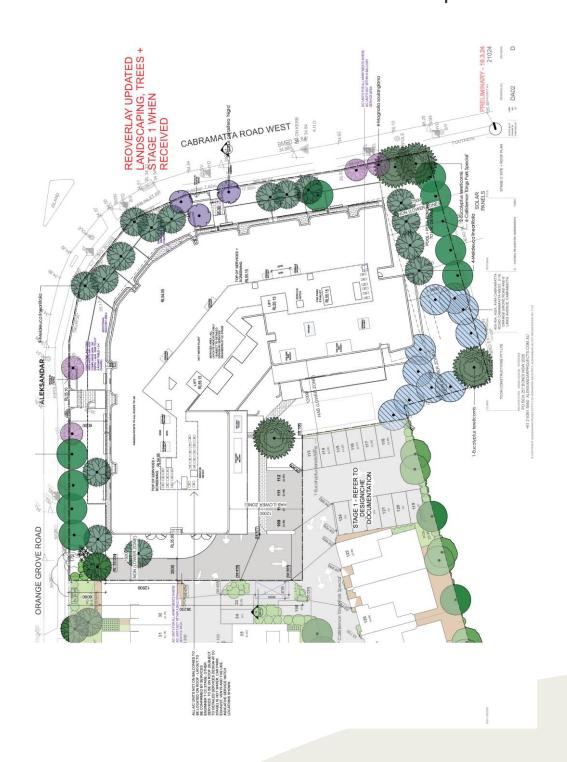
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<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

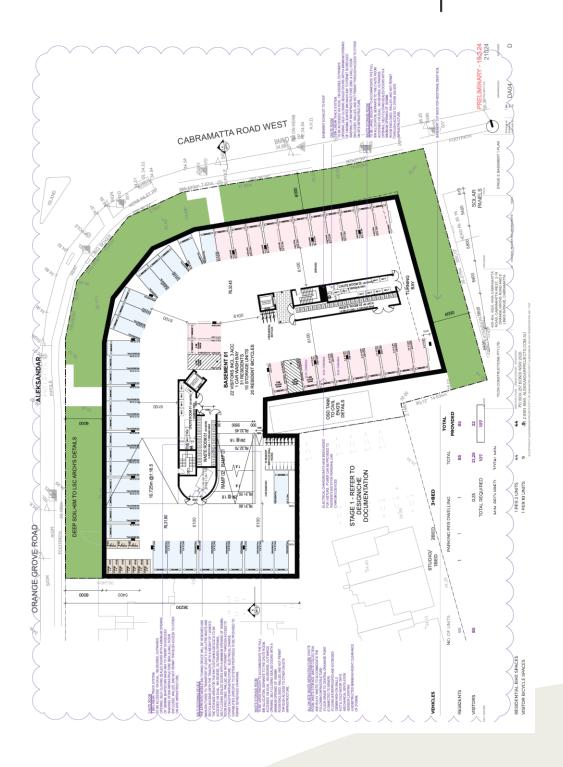
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Appendix C – Updated Vehicular Swept Paths (Townhouses)

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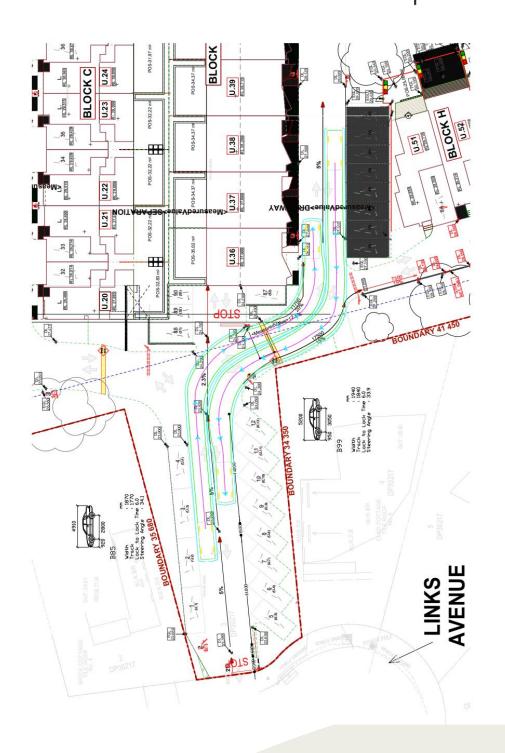
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<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

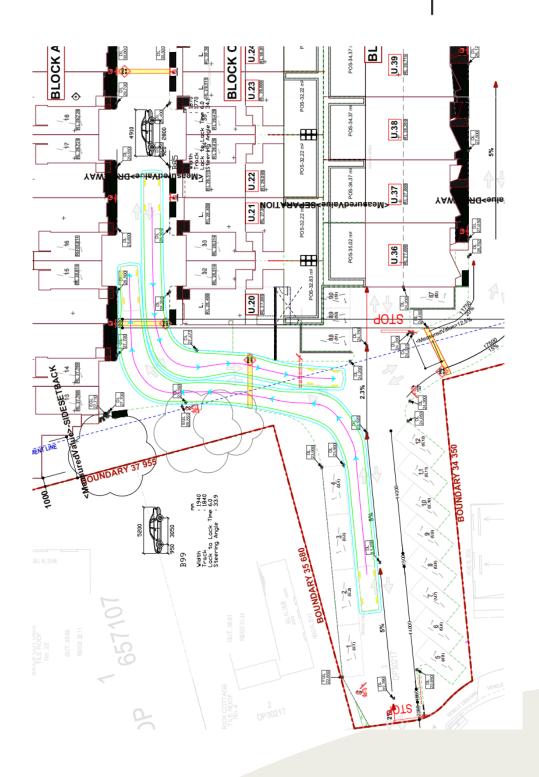
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<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

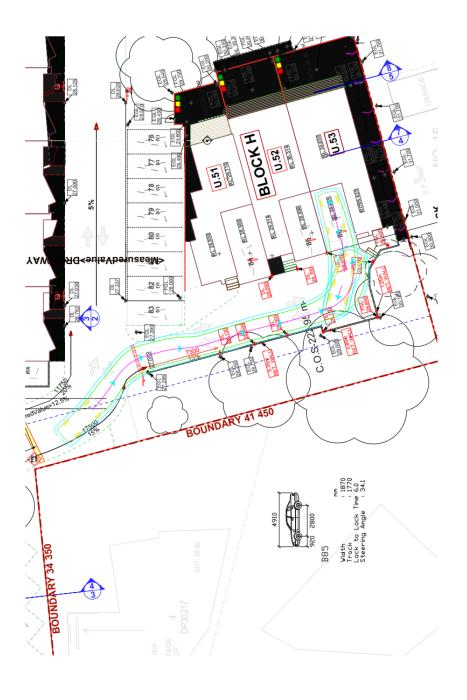
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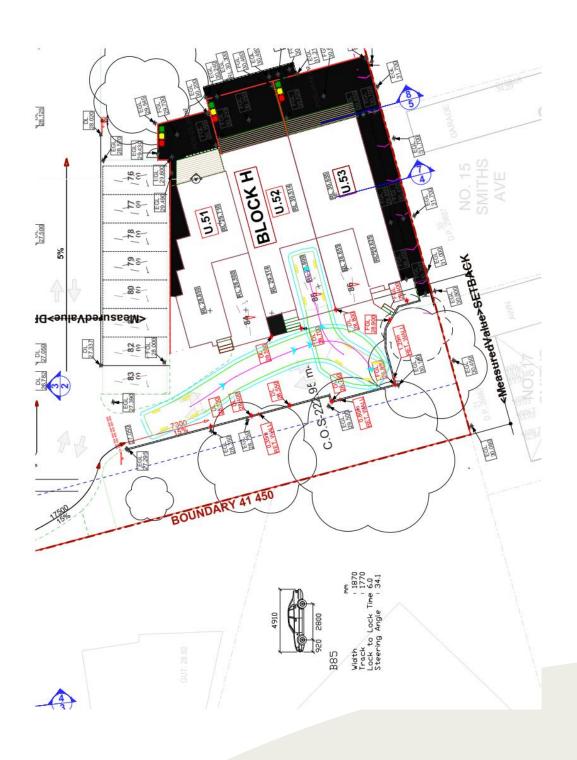
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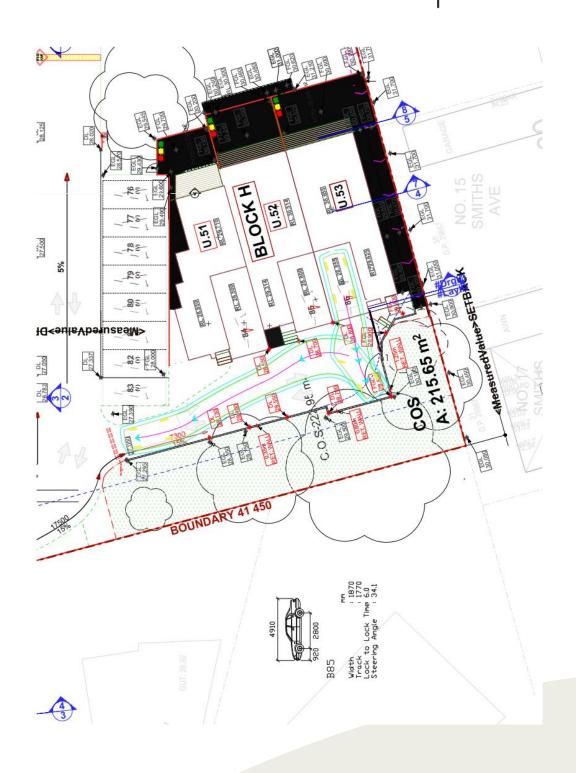
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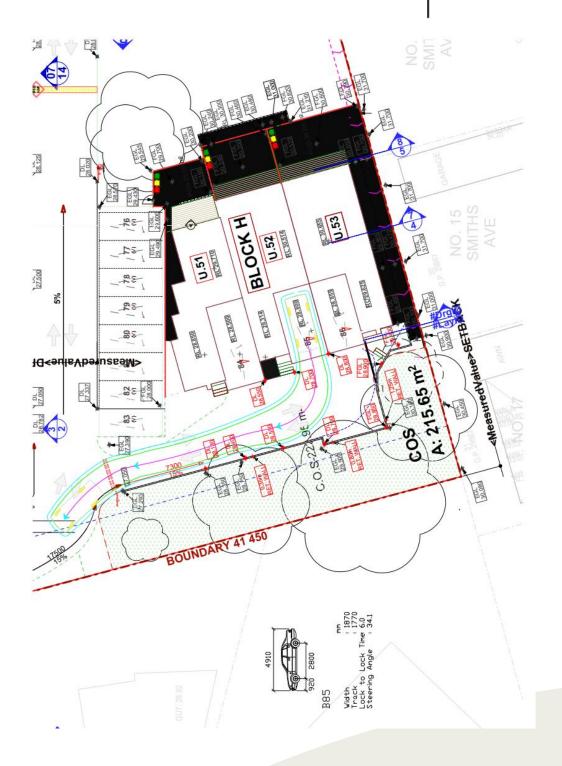
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Appendix D – Updated Vehicular Swept Paths (RFB)

<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

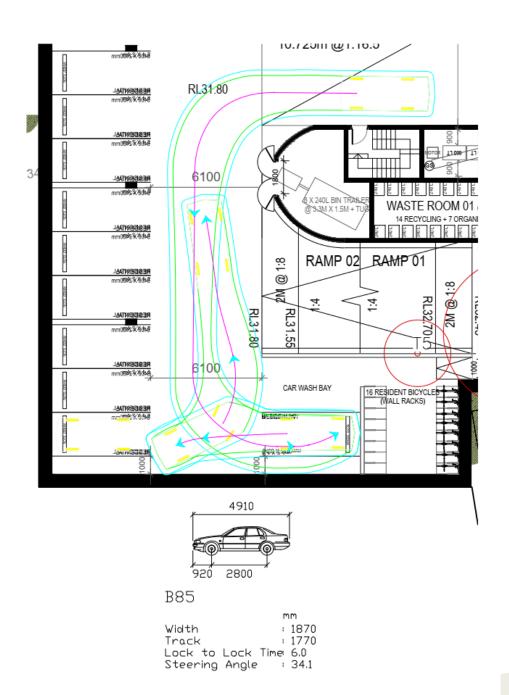
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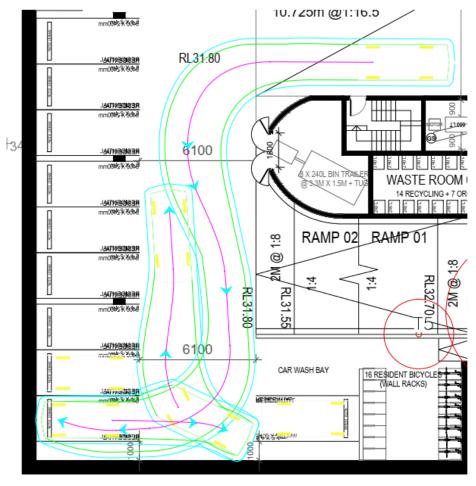
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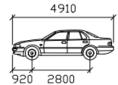
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B85

Width : 1870 Track : 1770 Lock to Lock Time 6.0 Steering Angle : 34.1

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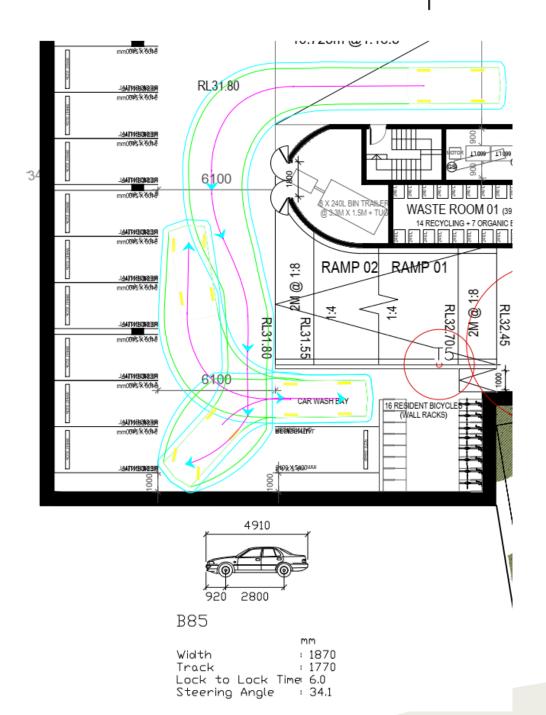
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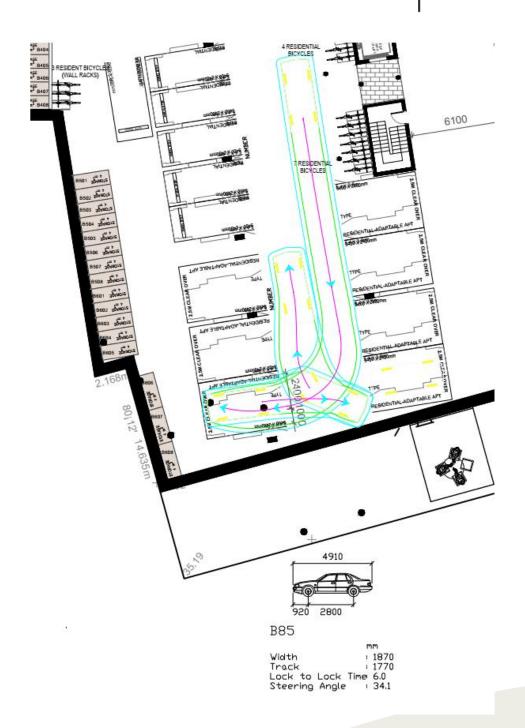
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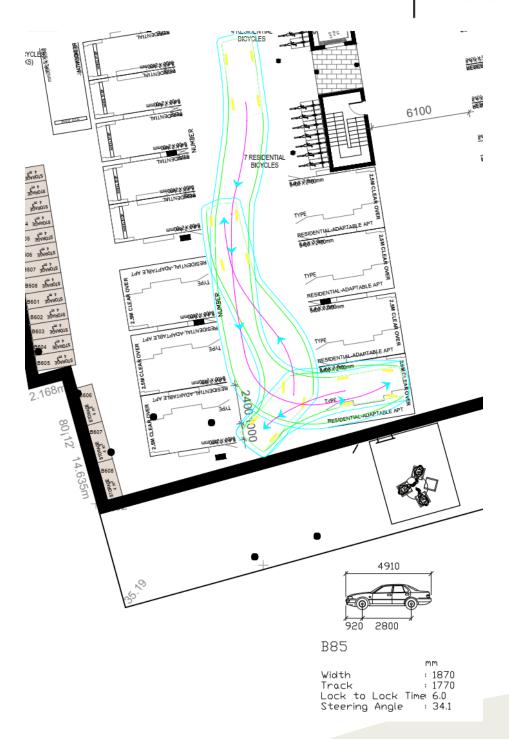
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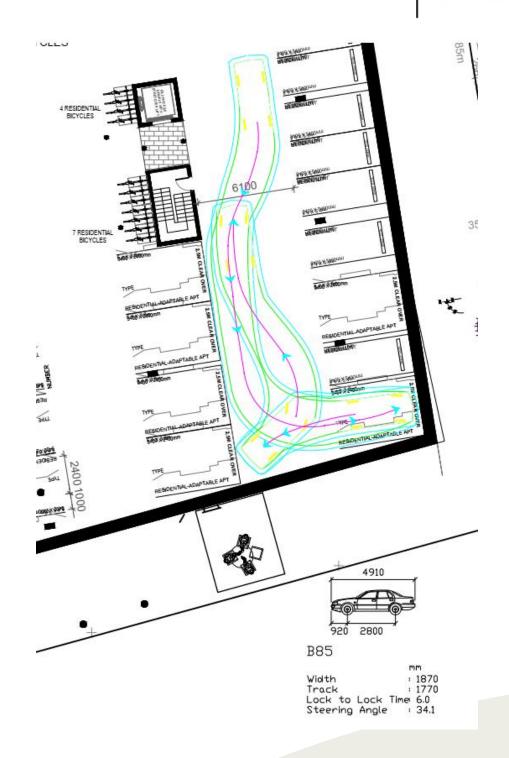
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Appendix E – Updated Vehicular Swept Paths (Heavy Vehicle access)

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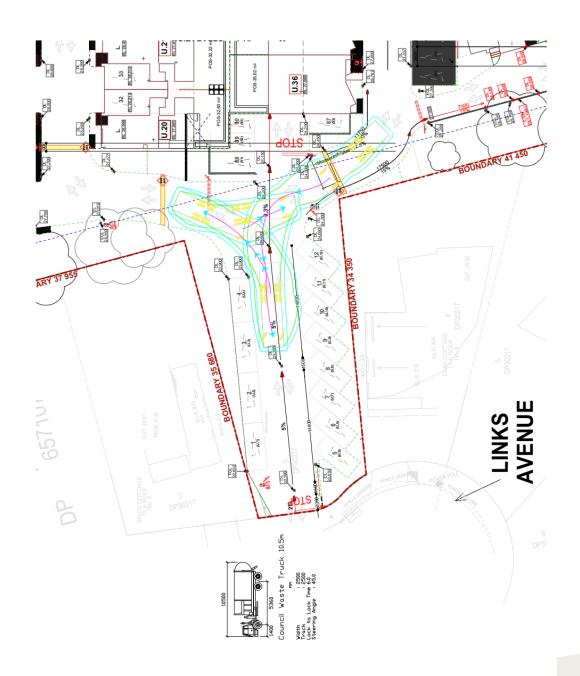
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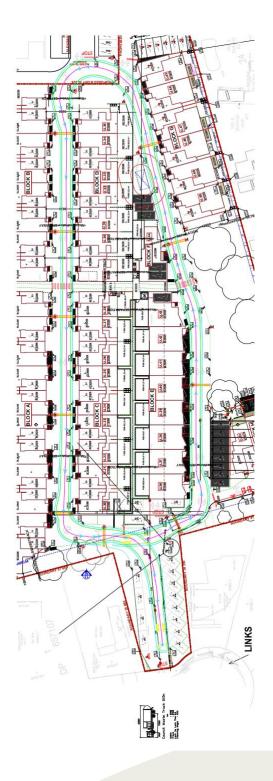
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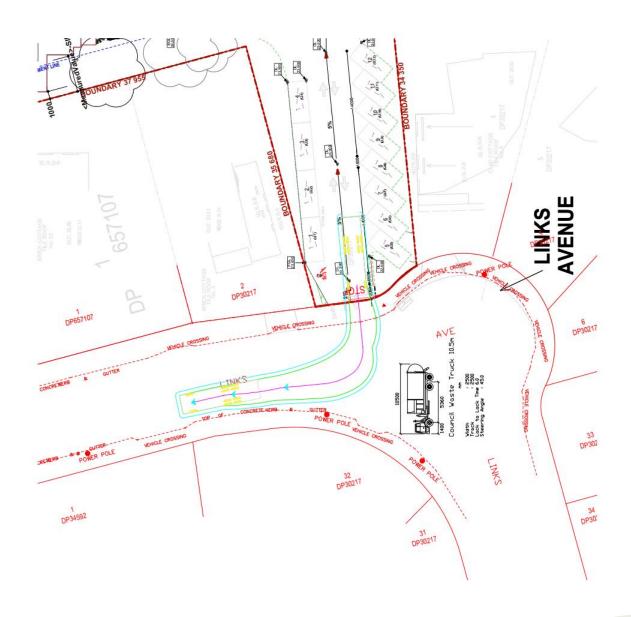
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Appendix F – Email From Transport for New South Wales (TfNSW)

<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

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E projects@hemanote.com.au



From: Brett Morrison (TAR TECHN) <Brett.Morrison4@transport.nsw.gov.au>

Sent: Thursday, March 7, 2024 1:08 PM

To: projects@hemanote.com.au; Development Sydney

<Development.Sydney@transport.nsw.gov.au>

Subject: RE: Seeking TfNSW feedback & Advice - 400-404 Cabramatta Road West, Cabramatta

- Proposed Multi Dwelling Housing & RFB

Hany,

TfNSW advises that current practice is to limit the number of vehicular conflict points along the arterial road network to maintain network safety and efficiency. This current practice is reflected in Section 6.2.1 of TfNSW current publication of the Guide to Traffic Generating Developments, which states 'access across the boundary with a major road is to be avoided wherever possible'.

Cabramatta Road and Orangegrove Roads are major arterial roads, which carries a high volume of traffic, where transport safety and efficiency of through traffic is of great importance.

It is also noted that any future development application would need to address clause 2.119 of *State Environmental Planning Policy (Infrastructure)* 2021, states:

"The consent authority must not grant consent to development on land that has frontage to a classified road unless it is satisfied that:

where practicable, vehicular access to the land is provided by a road other than the classified road".

With consideration of the above, TfNSW would require any future development of the site to not have vehicular access on the classified road frontage, with all vehicular access to be provided by via Links Road.

Note: It is emphasised that the comments provided above are informal and of a Pre-DA nature, they are not to be interpreted as binding upon Transport for New South Wales (TfNSW) and may change following formal assessment of a submitted development application from the appropriate consent authority.

Brett Morrison

Senior Land Use Planner Land Use Assessment Western Planning and Programs Greater Sydney

Transport for NSW

E Brett.Morrison4@transport.nsw.gov.au

4 Parramatta Square, 12 Darcy Street, Parramatta NSW 2150 PO Box 973 Parramatta CBD NSW 2124



Transport for NSW

Hemanote Consultants Pty Ltd
ABN 94 606 345 117

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Appendix G – Traffic Volume Surveys

<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

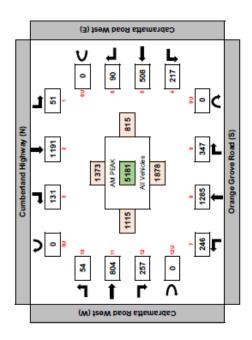
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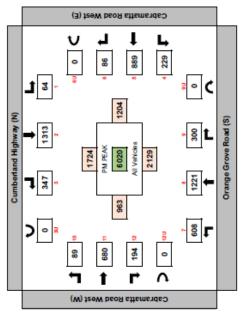
MOOREBANK NSW 1875

P 0414 251 845

E projects@hemanote.com.au

Consultants









Report Type:	Peak Hour Volume Diagram
Geocounts Job ID:	1705591913351
Client Job Number:	n/a
Client Name:	Hemanote Consultants
Location:	Cabramatta
Survey Site:	IC01 (Cumberland HwyCabramatta Rd WealfOrange Grove Rd)
Survey Date:	Thurs day, 22 February 2024
Site Coordinates:	-33.8994319, 150.9191707
AM Peak Hour:	7:45 to 8:45
PM Peak Hour:	16:30 to 17:30

Hemanote Consultants Pty Ltd ABN 94 606 345 117

PO Box 743

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Report Type:

Geocounts Job ID:

Client Job Number:

Client Name:

Location:

Cabramatta

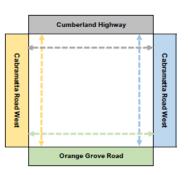
Curvey Site:

IC01 (Cumberland Hwy/Cabramatta Rd West/Orange Grove Rd)

Thursday, 22 February 2024

Site Coordinates:

-33.8994319, 150.9191707



	North	East	South	West	Total		
Peds Crossing AM	24	24 3 7 2 36					
Peds Crossing PM	22 19 12 6 59						
Peak Hour Peds AM	8:00 to 9:00						
Peak Hour Peds PM		15:00 to 16:00					

15min Peds

	Leg	ı	North	East	South	West	Total
6:30	to	6:45	1	0	0	0	1
6:45	to	7:00	0	0	0	0	0
7:00	to	7:15	1	1	0	0	2
7:15	to	7:30	0	0	0	0	0
7:30	to	7:45	2	0	2	0	4
7:45	to	8:00	0	0	0	0	0
8:00	to	8:15	4	1	0	0	5
8:15	to	8:30	4	0	1	1	6
8:30	to	8:45	8	1	0	0	9
8:45	to	9:00	3	0	3	0	6
9:00	to	9:15	0	0	0	0	0
9:15	to	9:30	1	0	1	1	3
14:30	to	14:45	2	0	0	0	2
14:45	to	15:00	0	0	1	0	1
15:00	to	15:15	10	5	0	3	18
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16:30	to	16:45	0	0	0	0	0
16:45	to	17:00	1	0	4	1	6
17:00	to	17:15	2	0	1	0	3
17:15	to	17:30	0	2	1	0	3
17:30	to	17:45	0	3	0	0	3
17:45	to	18:00	0	0	0	0	0
18:00	to	18:15	1	1	1	1	4
18:15	to	18:30	0	0	0	0	0

60min Peds

	Leg		North	East	South	West	Total
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6:45	to	7:45	3	1	2	0	6
7:00	to	8:00	3	1	2	0	6
7:15	to	8:15	6	1	2	0	9
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16:45	to	17:45	3	5	6	1	15
17:00	to	18:00	2	5	2	0	9
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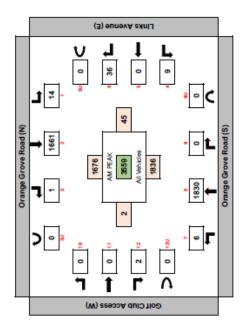
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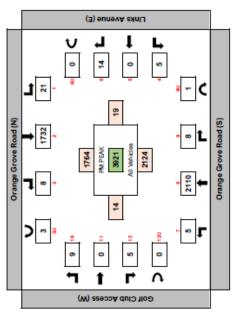
MOOREBANK NSW 1875

P 0414 251 845

E projects@hemanote.com.au

Consultants





Select Vehicle Type All Vehicles



Report Type:	Peak Hour Volume Diagram
Geocounts Job ID:	1705591913351
Client Job Number:	n/a
Client Name:	Hemanote Consultants
Location:	Cabramatta
Survey Site:	IO02 (Orange Grove Rdf.inks Ave/Goff Club Access)
Survey Date:	Thursday, 22 February 2024
Site Coordinates:	-33.9017472, 150.9185055
AM Peak Hour:	7:45 to 8:45
PM Peak Hour:	16:30 to 17:30

<u>Hemanote Consultants Pty Ltd</u> ABN 94 606 345 117

PO Box 743

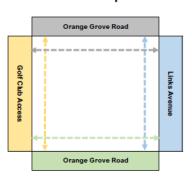
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Report Type: Pedestrian Data
Geocounts Job ID: 1705591913351
Client Job Number: Hemanote Consultants
Location: Cabramatta
Survey Site: IC02 (Orange Grove Rd/Links Ave/Golf Club Access)
Survey Date: Thursday, 22 February 2024
Site Coordinates: -33.9017472, 150.9185055



	North	East	South	West	Total	
Peds Crossing AM	1	13	0	0	14	
Peds Crossing PM	2 17 1 1 21					
Peak Hour Peds AM	7:30 to 8:30					
Peak Hour Peds PM	16:45 to 17:45					

15min Peds

Leg		North	East	South	West	Total	
6:30	to	6:45	0	0	0	0	0
6:45	to	7:00	0	1	0	0	1
7:00	to	7:15	0	0	0	0	0
7:15	to	7:30	0	0	0	0	0
7:30	to	7:45	0	3	0	0	3
7:45	to	8:00	0	2	0	0	2
8:00	to	8:15	1	1	0	0	2
8:15	to	8:30	0	2	0	0	2
8:30	to	8:45	0	2	0	0	2
8:45	to	9:00	0	0	0	0	0
9:00	to	9:15	0	0	0	0	0
9:15	to	9:30	0	2	0	0	2
14:30	to	14:45	0	1	0	0	1
14:45	to	15:00	0	1	0	0	1
15:00	to	15:15	0	0	0	0	0
15:15	to	15:30	0	1	0	0	1
15:30	to	15:45	0	2	0	0	2
15:45	to	16:00	0	3	0	0	3
16:00	to	16:15	0	0	0	0	0
16:15	to	16:30	0	2	0	0	2
16:30	to	16:45	0	0	0	0	0
16:45	to	17:00	0	1	0	0	1
17:00	to	17:15	0	0	0	0	0
17:15	to	17:30	0	2	0	0	2
17:30	to	17:45	2	3	1	1	7
17:45	to	18:00	0	1	0	0	1
18:00	to	18:15	0	0	0	0	0
18:15	to	18:30	0	0	0	0	0

60min Ped

	Leg		North	East	South	West	Total
6:30	to	7:30	0	1	0	0	1
6:45	to	7:45	0	4	0	0	4
7:00	to	8:00	0	5	0	0	5
7:15	to	8:15	1	6	0	0	7
7:30	to	8:30	1	8	0	0	9
7:45	to	8:45	1	7	0	0	8
8:00	to	9:00	1	5	0	0	6
8:15	to	9:15	0	4	0	0	4
8:30	to	9:30	0	4	0	0	4
14:30	to	15:30	0	3	0	0	3
14:45	to	15:45	0	4	0	0	4
15:00	to	16:00	0	6	0	0	6
15:15	to	16:15	0	6	0	0	6
15:30	to	16:30	0	7	0	0	7
15:45	to	16:45	0	5	0	0	5
16:00	to	17:00	0	3	0	0	3
16:15	to	17:15	0	3	0	0	3
16:30	to	17:30	0	3	0	0	3
16:45	to	17:45	2	6	1	1	10
17:00	to	18:00	2	6	1	1	10
17:15	to	18:15	2	6	1	1	10
17:30	to	18:30	2	4	1	1	8

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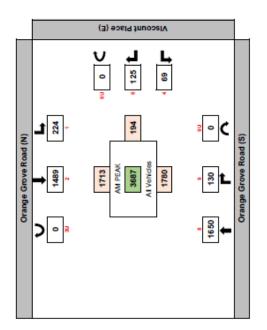
PO Box 743

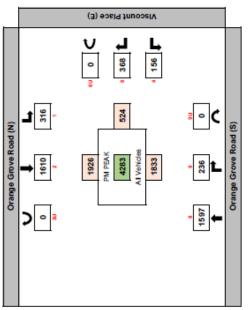
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Select Vehicle Typ All Vehicles



Report Type:	Peak Hour Volume Diagram
Geocounts Job ID:	1705591913351
Client Job Number:	n/a
Client Name:	Hemanote Consultants
Location:	Cabramatta
Survey Site:	IC03 (Orange Grove Rd/Viscount Place)
Survey Date:	Thursday, 22 February 2024
Site Coordinates:	-33.907878, 150.9186396
AM Peak Hour:	8:00 to 9:00
PM Peak Hour:	15:30 to 16:30

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Report Type: Pedestrian Data

Geocounts Job ID: 1705591913351

Client Job Number: n/a

Client Name: Hemanote Consultants

Location: Cabramatta

Survey Site: IC03 (Orange Grove Rd/Viscount Place)

Survey Date: Thursday, 22 February 2024

Site Coordinates: -33.907878, 150.9186396



	North	East	South	Total		
Darla Cassaira AM	0	4		4		
Peds Crossing AM	0	4	0	4		
Peds Crossing PM	0	18	9	27		
Peak Hour Peds AM	7:30 to 8:30					
Peak Hour Peds PM		15:15 t	o 16:15			

15min Peds

	Leg		North	East	South	
6:30	to	6:45	0	1	0	1
6:45	to	7:00	0	0	0	0
7:00	to	7:15	0	0	0	0
7:15	to	7:30	0	0	0	0
7:30	to	7:45	0	1	0	1
7:45	to	8:00	0	1	0	1
8:00	to	8:15	0	0	0	0
8:15	to	8:30	0	1	0	1
8:30	to	8:45	0	0	0	0
8:45	to	9:00	0	0	0	0
9:00	to	9:15	0	0	0	0
9:15	to	9:30	0	0	0	0
14:30	to	14:45	0	2	0	2
14:45	to	15:00	0	1	0	1
15:00	to	15:15	0	0	2	2
15:15	to	15:30	0	2	0	2
15:30	to	15:45	0	0	0	0
15:45	to	16:00	0	2	1	3
16:00	to	16:15	0	6	4	10
16:15	to	16:30	0	0	0	0
16:30	to	16:45	0	0	0	0
16:45	to	17:00	0	1	0	1
17:00	to	17:15	0	2	0	2
17:15	to	17:30	0	1	0	1
17:30	to	17:45	0	0	0	0
17:45	to	18:00	0	0	2	2
18:00	to	18:15	0	0	0	0
18:15	to	18:30	0	1	0	1

60min Peds

	_					
	Leg		North	East	South	Total
6:30	to	7:30	0	1	0	1
6:45	to	7:45	0	1	0	1
7:00	to	8:00	0	2	0	2
7:15	to	8:15	0	2	0	2
7:30	to	8:30	0	3	0	3
7:45	to	8:45	0	2	0	2
8:00	to	9:00	0	1	0	1
8:15	to	9:15	0	1	0	1
8:30	to	9:30	0	0	0	0
14:30	to	15:30	0	5	2	7
14:45	to	15:45	0	3	2	5
15:00	to	16:00	0	4	3	7
15:15	to	16:15	0	10	5	15
15:30	to	16:30	0	8	5	13
15:45	to	16:45	0	8	5	13
16:00	to	17:00	0	7	4	11
16:15	to	17:15	0	3	0	3
16:30	to	17:30	0	4	0	4
16:45	to	17:45	0	4	0	4
17:00	to	18:00	0	3	2	5
17:15	to	18:15	0	1	2	3
17:30	to	18:30	0	1	2	3

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Appendix H – SIDRA Base Model Calibration

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Cumberland Hwy/Orange Grove Rd/Cabramatta Rd W Signalised Intersection Queue (number of vehicles) recorded at start of green light phase for each lane

AM Peak (7:45AM - 8:45AM) Field Approach Lane Direction Average 95th North 1 left 0.5	Field n Average 0.5
thru 9.4	
thru 14.7	
thru 15.1	
right 4.9	
left 3.3	
thru 8.9	
thru 8.7	
right 3.7	
left 1.2	
thru 6.5	
thru 13.6	
thru 16.9	
right 14.3	
left/thru 30+	
thru 30+	
right 5.4	
right 5.3	

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Cumberland Hwy/Orange Grove Rd/Cabramatta Rd W Signalised Intersection

PM Peak	(4:30PM	PM Peak (4:30PM - 5:30PM)						
			Field		SIDRA-base		SIDRA-Calibrated	ted Comments
Approach Lane	Lane	Direction	Average 95th	th	Average 95th	th	Average 95th	th
North	1	left	0.5	1.8				
North	2	thru	8.5	12.7	15.5	25.2	11.9	19.4 80% north through deman
North	3	thru	14.8	17.0	14.3	23.3	11	17.9 80% north through deman
North	4	thru	14.4	17.7	14.3	23.3	11	17.9 80% north through deman
North	5	right	14.7	17.0	24.3	39.6	11.2	18.4 80% north right turn dema
East	1	left	4.8	8.2	3.4	5.6	3.4	5.6
East	2	thru	29.7	32.0	17.9	29.3	17.9	29.3
East	æ	thru	29.6	32.0	17.9	29.3	17.9	29.3
East	4	right	4.1	8.7	3.6	5.9	3.6	5.9
South	1	left	4.0	7.0				
South	2	thru	28.1	40.0	26.9	43.8	26.9	43.8
South	æ	thru	24.2	40.0	22.8	37.2	22.8	37.2
South	4	thru	27.0	40.0		37		37
South	2	right	15.7	25.0	13.4	21.8	13.4	21.8
West	1	left/thru	16.0	25.0	13.1	21.4	13.1	21.4
West	2	thru	15.7	24.4	13	21.2	13	21.2
West	m	right	4.6	7.0	4.1	6.7	4.1	6.7
West	4	right	3.6	5.0	4.1	6.7	4.1	6.7

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demand demand demand demand

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Orange Grove Rd/Links Ave/Golf Club Access Signalised Intersection Queue (number of vehicles) recorded at start of green light phase for each la

Queue (nu	umber of v	ehicles) recc	orded at start of	green	Queue (number of vehicles) recorded at start of green light phase for each lane	th lane		
AM Peak	AM Peak (7:45AM - 8:45AM)	8:45AM)						
			Field		SIDRA-base	SIDR	SIDRA-Calibrated	Comments
Approach Lane	Lane	Direction	Direction Average 95th		Average 95th	Aver	Average 95th	
North	1	left/thru	0.1	0.7	5.6	9.2	1.4	2.3
North	2	thru	1.2	2.4	5.7	9.2	1.4	2.4 25% north through of
North	3	thru	0.3	1.4	5.7	9.2	1.4	2.4 25% north through of
North	2	right	0.0	0.0	0	0.1	0	0.1
East	4	left/right	2.0	4.0	1.9	3.1	1.9	3.1
South	1	left/thru	1.2	5.0	6.2	10	4 6	6.5
South	2	thru	1.7	6.0	6.2 1	10.1	4	6.6 65% south through
South	33	thru	2.4	7.1	6.2 1	10.1	4	6.6 65% south through
South	4	right	0.0	0.0	0	0.1	0	0.1
West	1	left/right	1.0	1.0	0.1	0.2	0.1	0.2

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Orange Grove Rd/Links Ave/Golf Club Access Signalised Intersection

PM Peak (4:30PM - 5:30PM)	4:30PM	- 5:30PM)							
			Field		SIDRA-base		SIDRA-Calibrated	d Comments	ents
Approach Lane	Lane	Direction	Direction Average 95th	th.	Average 95th	95th	Average 95th		
North	1	left/thru	0.2	1.0	6.3	10.2	1.6	2.6	
North	2	thru	1.4	2.4	6.3	10.2	1.6	2.6 25% no	2.6 25% north thorugh demand
North	3	thru	9.0	2.0	6.3	10.2	1.6	2.6 25% no	2.6 25% north thorugh demand
North	2	right	0.0	0.0	0.4	9.0	0.4	9.0	
East	4	left/right	1.4	2.3	8.0	1.4	1.4	2.3 170%	2.3 170% west through demand
South	1	left/thru	3.7	7.6	21.5	35.1	9.5	9.1	
South	2	thru	4.2	9.3	22.1	36	5.8	9.4 75% sc	9.4 75% south thorugh demand
South	3	thru	4.6	10.6	22.2	36.2	5.8	9.5 75% sc	9.5 75% south through demand
South	4	right	0.3	1.0	0.4	0.6	0.4	9.0	
West	1	left/right	1.1	2.2	9.0	1	1.2	1.9 200%	1.9 200% west through demand

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Orange Grove Rd/Viscount Place Signalised Intersection Queue (number of vehicles) recorded at start of green light phase for each

Queue (nu	mber of v	ehicles) reco	Queue (number of vehicles) recorded at start of green light phase for each lane	of green	light phase f	or each l	ane		
AM Peak (7:45AM - 8:45AM)	(7:45AM -	.8:45AM)							
			Field		SIDRA-base		SIDRA-Calibrated	d Comments	
Approach Lane	Lane	Direction	Direction Average 95th	5th	Average 95th	5th	Average 95th		
North	1	left/thru	1.7	3.0	1.1	1.8	1.5	2.4 Use Calculated PFF 0.699	
North	2	thru	11.3	20.5	10	16.2	10.5	17.1 Use Calculated PFF 0.699	_
North	3	thru	11.3	22.3	10.1	16.4	11.3	18.4 Use Calculated PFF 0.699	_
East	1	left	2.2	4.0	0.7	1.2	2.1	3.5 Use Calculated PFF 0.742	
East	2	right	1.8	3.8	1	1.6	2.4	4 Use Calculated PFF 0.742	
East	3	right	2.4	4.8	1	1.6	2.4	4 Use Calculated PFF 0.742	
South	1	thru	3.0	7.9	5.1	8.3	6.9	11.2 Use Calculated PFF 0.698	
South	2	thru	5.4	12.9	5.1	8.3	6.7	10.9 Use Calculated PFF 0.698	••
South	3	right	2.7	5.8	1.1	1.7	2.9	4.7 Use Calculated PFF 0.698	••
South	4	right	2.2	4.8	1.1	1.7	2.9	4.7 Use Calculated PFF 0.698	

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Orange Grove Rd/Viscount Place Signalised Intersection Queue (number of vehicles) recorded at start of green light phase for each lane

	SIDRA-base SIDRA-Calibrated Comments	Average 95th Average 95th	1.7 2.8 3 4.9 140% north left turn demand	9 14.7 3.7 6.1 35% north through demand	9 14.7 3.7 6.1 35% north through demand	2.3 3.7 4.9 7.9 400% east left turn demand	3.4 5.5 5 8.1 200% east right turn demand	3.4 5.5 8.1 200% east right turn demand	5.6 9.2 2.1 3.5 30% south through demand	5.6 9.2 2.1 3.5 30% south through demand	1.8 2.9 4 6.6 290% south right turn demand	
	S		2.0	5.7	0.9	8.0	8.7	6.7	3.0	4.7	6.7	
	Field	Direction Average 95th	2.1	2.3	1.9	5.0	6.4	6.7	1.4	2.4	4.0	
5:30PM)		Direction	left/thru	thru	thru	left	right	right	thru	thru	right	
4:30PM -		Lane	1	2	3	1	2	3	1	2	3	
PM Peak (4:30PM - 5:30PM)		Approach Lane	North	North	North	East	East	East	South	South	South	

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Appendix I – SIDRA Intersection Analysis Results

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MOVEMENT SUMMARY - 7.45am - 8.45am - Base Year 2024 - Cumberland Highway / Cabramatta Road West / Orange Grove Road

Vehicle Movema	nt Performa	90										Veh	cle Move	ment Pe	rformar	nce									
Mov Tam Mov	Total Posses	American Total HV	S S	Aver Delay	Level of Service	95% Back Of Garian [Ven Det] voh m	Of Owner	g å	E Se al	A Se all	Anax Speed kmth	M _O	Turn Mov Class		Demand Flows (Total HV)	Armonal Flows Total HV	Sam A	Annua Delay	Lend of Service		95% Back Of Cassos Prop. Over. Det J. ver. m.	go –	E g j	No. of Cycles	Speed
South, Cumberland	Camberland Hey (240m)											South:	Cumber	Cumberland Hwy (240m)	(240m)										
1 L2 AEMOs 2 T1 AEMOs		259 2.4 1353 7.9		4.7		313	229.4	060	0.03	0.90	25.2	1 2	12 A		266 24	266 24 1364 7.8	0.731	72	LOSD	31.9	2303	3 090	0.84	0.90	36.3
3 R2 All MCs Approach	1012 63		1,000	53.6	LOSD	313	152.9	0.92	1.24	1.02	28.3	3 Apple		All MCs 2	220 0.8	220					31			1.07	262
East Cabranutta Rd W (500m)	(MODE) M PR											East		Cabramatta Rd W (500m)	(500m)										
4 L2 AIMOs		228		14.3	LOSA	6.9	42.8		0.68	0.45	41.2	4	L2 AL	All MCs 2	34 60	234								0.45	41.1
5 TI AIMOS	3 535 45			49.4	DSOT	16.0	116.0	0.93	0.79	0.93	282	un o	F 8		536 4.5	536 45	0.565	49.4	0807	16.0	116.0	0 0 0 0	0.79	0.93	282
		858	909'0	42.9	1080	16.0	116.0		0.78	0.81	566	Approach			963 4.4	963 4.4		100						0.81	29.9
North: Cumberland	Cumberland Hwy (500m)											North	Cumber	Cumberland Hwy (500m)	(600m)										
7 L2 AIMOs	28 54 78	54 78		24.7	10SB	72.4	170.1	0.83	0.83	0.83	36.4	7		All MCs	84 78	54 78			108B					0.84	36.4
8 T1 AIMOS		1254	0.599	38.5	LOSC	23.1	175.7	98'0	0.78	0.85	29.3	00	T A		257 10.0	-		38.6	_	232	176.2	2 0.85		0.85	292
9 R2 All MCs	Cs 136 3.8	136 3.8	0.763	77.7	100 F	9.6	70.8	1.00	0.88	1.13	21.9	6		All MCs	138 3.8				LOSF					1.13	
Approach	1445 9.3	1445	0.763	41.8	2801	177	175.7	9970	0.79	0.87	28.5	Appr	Approach	4	148 9.3	1448							0.79	0.87	
West Cabramatta Rd	Ru W (500m)											West	Cabramatta Rd	afta Rd VA	W (500m)										
2 :		88		85.4	LOSF	46.3	332.1	1.00	128	145	671	10			57 7.4	8								1.45	17.9
TI TI ALIMOS TO RO ALIMOS		271 4.7	*0.984	90.0	LOSF	10.3	75.0	1.00	136	132	6.3	12	F 28	All MCs 2	273 46	273 46	* 0.984	118.4	LOSF	10.4	332.3	3 100	52 68	1,46	18.0
Approach	1174 3.0	2	7860	110.8	108 F	46.3	332.1	100	122	143	15.8	Appr			176 30	1196								1.43	15.8
All Vehicles	1.9 6875	5309 6.1	1.088	614	108E	46.3	332.1	06.0	160	1.04	24.5	All	All Vehicles	36	6338 6.0	5358 6.0	1.190	65.2	LOSE	463	3703	3 050	160	106	23.6

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MOVEMENT SUMMARY - 7.45am - 8.45am - Base Year 2024 - Orange Grove Road / Links Avenue / Golf Club Access

				ב	e-	e v	dole	rre-Development	_									ĭ	-150	Dev	Post-Development	эше	=			
Š	S4 634	d memer	doman											2487	Vehicle Movement Performance	0	diago					- 1	- 1		- 1	- 1
20	v Tum Mov Class		Market Market	Annual Park	See a	Aner Li	Server S	95% Back Of Queeze Pop Que (Veh Dist)	(Ost)		五月月	S and a	17 1	M _O	Tum Mov Class	> 8 - 10 - 10 - 10 - 10 - 10 - 10 - 10 - 10	Demand Flows odal IN J Tr th % vel	American Figures 17	Saffin y	Aver Unday 5	Lavel of Service	36% Back Of Qual. [Net. Diet] veh m		g å	E S	No. of Cycles
8	uth: Cumb	South: Cumberland Hay (670m)	(670m)											South:	h; Cumbort	Cumborland Hwy (670m)	(M0Z)									
-	12	All MCs	616.7	616.7	0.304		LOSA	8.5	62.5	0.33	0.30	0.33	53.5	-		All MCs 6		6167	0.312	12.3	LOSA	9.0	66.3	0.36	0.32	0.36
N	2 TI A	MCs 12			0.304	5.3	LOSA	8.6	63.1				7.0%	CH	TI All !			1252 5.8	-0.312	6.9	LOSA	9.5	67.3	0.36	0.31	0.35
2	R2 A	MCs	00	1 00	0.014		LOSF	0.1	90	0.97	0.59	0.97	26.8	0		All MCs 3	0.0	3 00	* 0.042	19.1	1007	0.2	1.6	980	063	980
Ap	proach				0.304		PSOT	9 00	63.1				90'6	Appn	Approach	1262	6.8	62 58	0,312	6.1	LOSA	82	67.3	0.36	0.31	0.35
East		Links Ave (360m)												East	Links Ave (360m	(360m)										
4		All MCs	00 6		0.346		TOSE	32	23.6				13.8	4		All MCs 19	0.0	19 0.0	0.520		LOSE	6.6	46.4	680	0.78	68'0
10	I		1 00	1 00 *	• 0.346	59.4	LOSE	3.2	23.6	96.0	0.75		18.4	9	TI All			1 0.0	*0.520	69.4	LOSE	6.6	46.4	66.0	97.0	66.0
10		All MCs	39 66		0.346		1001	3.2	23.6			080	13.6	9			1 28	76 28	0.520		LOSF	6.6	46.4	660	0.78	660
App	Approach		48 43		0.346		LOSF	3.2	22.6				13.9	Appn		96	3 2.2	96 22	0.520		LOSF	6.5	46.4	66.0	0.78	66.0
No	ŧ	Cumberland Hwy (240m	(240m)											North	r Cumberla	Cumberland Hwy (240m)	4000									
1	77	All MCs	15143	1514.3	0.107	11.3	LOSA	2.5	19.1				671	7		All MCs 25	25 8.3	25 8.3	0.111	11.2	LOSA	2.8	20.7	0.29	0.31	0.29
00	F	All MCs 4		437 8.1	0.107	4.5	LOSA	2.6	19.2				51.6	100					0.111	5.0	LOSA	2.8	20.9	0.29	0.26	0.29
6	R2	All MCs	1 00		*0.013	77.5	LOSF	0.1	90	260	65.0	26.0	15.4	6	R2 All	All MCs 1	1 0.0	1 0.0	0.013	78.6	LOSF	0.1	9.0	96'0	650	96.0
App	Approach	4	483 83	453 83	0.107	4.9	LOSA	2.6	19.2				808	Appn	Approach	463	8.1	163 8.1	0.111	5.5	LOSA	2.8	20.9	0.29	0.27	0.29
We	7	Parking Access (2)	(200ml)											West	West. Parking Access	ccess (200m)	(m)									
10	77	All MCs	1 00	1 00	0.023	618	LOSE	0.2	1.8				9.6	0		All MCs 1	0.0	1 0.0	0.000	144.1	LOSF	0.3	2.7	000	0.00	0.00
Ξ	F	All MCs	1 0.0	1 0.0	0.023	1.99	LOSE	0.2	1.8	0.83	0.62	0.83	19.3	=	TI All		1 0.0	1 0.0	0.000	161.3	LOSF	0.3	2.7	000	000	000
12	22	All MCs	1 100	1 100	0.023		LOSF	0.2	1.8				9'6	15		All MCs 1	100	1 100	00000	9.071	LOGF	0.3	2.7	000	000	000
App	Approach		3333	333.3	0.023	6.89	TOSE	0.2	1.8	0.93	0.62	0.93	13.4	Approach	pach	69	333.3	3333	0000	158.7	100 F	0.3	2.7	000	000	000
N	Vehicles	10	1764 65 1	1784 6.5	0.346	7.2	LOSA	9.8	63.1	0.34	030	0.34	9 55 6	All Ve	All Vehicles	1824	63	1824 63	0.520	6.7	LOSA	9.2	673	000	000	00.0

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MOVEMENT SUMMARY - 7.45am - 8.45am - Base Year 2024 - Orange Grove Road / Viscount Place

		-	re-L)ev	Pre-Development	mer	<u></u>				100				nes.	Pos	t-D	sve	Post-Development	Jent				
Vehicle Movement Perform		auce										Vehicle Movement Performance	ement	Performs	Suce									
Mov Turn Mov ID Class	BILL BUILD	Anival Flows (Total HV)	Safety %	Aver Delay	Avar. Laval of Select Delay Service	95% Back	95% Back Of Queue Prop. Case Veh. Dist veh. m		五の計	Avec. A No. of Sp Cycles Sp	Awar	Mov Turn Mov ID Class	- ×	Demand Flows Total HV 10			Deg Aver Saftn Delay vic gen	War Level of elay Service		% Back Of	95% Back Of Queue Prop Que Veh. Det veh. m	医房屋	No. of	Speed
South: Orange Grove Road	we Road											South: Orange	Orange Grove Road	Road								ı	ı	
2 T1 All MCs 2434 6.1 .	3 2434 6.1	434	0.786	5.8	LOSA	32.0	235.9			0.64	45.9	I	All MCs	2436 6.1	2436	6.1	0.787 5	5.8 LOS		32.1 2	236.5 0.54	94 0.49	0.54	46.9
3 R2 All MC	\$ 166 1.7	166 1.7	* 0.835	707	LOSF	9.0	35.6	1.00	0.92	1.37	23.4		All MCs	166 1.7	166	17 -03	* 0.836 70	70.7 LOS	u.		36.6 1,00	00 0.92	1.37	7 23.4
Approach	2600 5.8	2600 5.8	0.836	10.0	LOSA	32.0	235.9	0.57		0.69	38.4	Approach		2602 5.8	2602	5.8 0.	0.836 10	10.0 LOSA		32.1 2	236.5 0.57	7 0.52	0.59	38.
East Viscount Place	90											East Viscount Place	it Place											
4 L2 All MCs	\$ 77.35	77 3.5	0.245	48.6	LOSD	3.6	26.1	06.0	0.76	0.90	28.2	4 L2 A	All MCs	77 35	2 77	35 0	0.245 48	48.6 LOSD		36	26.1 0.90	0.76	060	787
6 R2 All MOs 147 3.7	35 147 3.7	147 3.7	*0.644	64.5	LOSE	42	30.1	1.00	0.81	1.10	19.7		All MCs	147 3.7	147		+0.644 64	64.5 LOSE			30.1 1.00	18.0 0.81	1.10	19.7
Approach	224 3.6	224 3.6	0.644	99.0	LOSE	42	30.1	76.0	0.80	1.03	22.6	Approach		224 3.6	5 224	36 0	0.644 59	3801 065		42	30.1 0.97	080 4	103	3 22 6
North: Orange Grove Road	we Road											North: Orange	Orange Grove F	Road										
7 L2 All MCs 283 10	3 283 10	283	0.185	611	LOSA	29	202	0.21		0.21 5	52.5		All MCs	283 1.0	0 283	1.0 0.1	0.196 12	120 LOSA		29 2	202 021	12 0.64	1 0.21	52.5
8 T1 All MCs 2152 92	3 2152 92	2152	*0.864	17.7	LOSB	47.4	358.7	0.81		780	47.0	8 T1 A	All MCs 2	2161 9.1	2161	9.1 +0.868		18.3 LOSB		48.4 3	365.5 0.92	2 0.78	9 0.05	5 46.6
Approach	2435 82	2435 8.2	0.854	17.0	LOSB	47.4	368.2	0.74	0.75	0.77	46.2	Approach		2444 82	2444	82 0	0.868 17	17.6 LOSB		48.4 3	366.5 0.75	92 0 92	8 0.78	3 45.8
Al Vehicles	5,759 6.8	5259 6.8	0.864	15.3	ROSB	47.4	368.7	190	190	69'0	42.3	All Vehicles		6270 68	5270	6.8 0	0.958 15	156 1088		48.4 3	365.5 0.67	72 0.64	0.70	1 42 1

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MOVEMENT SUMMARY – 4.30pm – 5.30pm – Base Year 2024 – Cumberland Highway / Cabramatta Road West / Orange Grove Road

				í													•		2	5						
ehicle Moveme	ent Perform	ance											Vehicie	Moveme	nt Pers	ormance	1000				_					
ov Tum Mov	Demand Flows [Total HV] [To	4.5	Plowes 1	See A	Avent Lea Dellay Se succ	Service 95	5% Eack Of Own [Veh Det] voh m	Det 1	Pop Pop	Egg	Anar No of S Cycles	W day	No.	Olass Class	8 g	Amond Flower Main PV To		Dog S	A Partie	Service Service		95% Back Of Guouse Prop. Case (Veh. Dest.) vish m	8 - Eg	### ### ### ##########################	O STATE OF THE PERSON NAMED IN	
South: Cumberland Hwy (240m)	nd Hwy (240m	1											South: C	South: Cumberland Hwy (240m)	Hwy (2	40m)										
L2 All MCs	640	9	33					391.7	1.00	1.16	133	14.5	-			642 33		0.995		_						
T1 All MCs		1285	5.7	*0.992 11	112.5 L		542	391.7	8 8	1.24	138	21.0	CH CT	T1 AIMCs R2 AIMCs			321 07	* 0.995	1026	100	283	391.7	001 0	125	2.3	186
	2241 43	2241	43			LOSF		391.7	8 8	1.19	137	19.1	Approach		2252	43		0.995								
East, Cabramatta Rd	Rd W (500m)												East Ca	Cabramatta Rd W (500m)	3d W (50	(mg)										
4 L2 All MCs	Cs 241 1.7	77	11	0.209		LOSA		39.3	0.40	190	0.40	127	4			1.6		0.227	12.8			43.6				
T1 All MCs		936	24			LOSF	39.2	280.1	1.00	1.17	131	21.0	9	TI AIMCS		2.4	936 2.4	*0.957	81.4		39.2	280.1	1 100	1.17	1,31	21.0
6 R2 All MCs	Ce 91 23	16	23		74.7 L	LOSF		443	1.00	67.0	101	27.3	9	32 All MCs	16 9	23		0.584		108 F		44				
Approach	1267 22	1367		9 296.0		TOSE	39.2	280.1	68.0	1.05	1.12	232	Approach	2	1287	22 1		0.957				280				
North: Cumberland	d Hwy (500m												North C	Cumberland Hwy		(500m)										
LZ All MCs			6.3		25.8 LI	E SOT		9 091	0.91	68.0	160	33.4	_			6.3		0.663		LOS B		162				
T1 All MCs		4 1106	4.4	0.657 4	_		22.5	163.2	0.95	0.83	0.92	25.6	8	T1 All MCs	-	4.4	1117 4.4	0.663	48.4		22.7	166.2	2 0.93	3 0.84	0.93	25.6
R2 All MCs	Cs 292 20		2.0			LOSF		159.2	1.00	660	125	21.4	6	\$2 All MGs	50 285 50 285	20		968'0		100 F		159				
Approach	1452 41		4.0					163.2	0.94	0.87	560	24.8	Approach	e	1463	4.0		0.898				168				
West Cabramana Rd W	Rd W (500m)												West C	Cabramatta Rd W (500m)	Rd W (5	(mgg										
10 L2 ABMCs	Cs 94 22		22	0.825 3	17	OSC	27.9	199.2	1.00	0.93	1.08	M	10 12			22	94 22	0.826								
11 TI AI MCs		216	52		6.09 L		27.9	199.2	1.00	96.0	1.08	25.9	=			2.5	716 2.5	0.026	600	LOSE	27.9	188.2	2 100	0.94	1.08	523
R2 All MCs	200	204	5.6				7.1	608	1.00	0.83	106	9.1	12	32 All MCs	213	2.5	113 2.5	* 0.687								
proach	1014 25	1014	2.5	0.825 6	F-	LOSE	6.12	199.2	1 00	0.92	1.08	22.6	Approac	£	1022	2.5	122 2.5	0.826		LOSE		***				
All Vehicles	5974 3.5	5 5974 3.5		0.992 7	727 1	LOSF	542	391.7	96.0	1.04	1.17	21.6	All Vehicles	los	6024	3.5	6024 3.5	966'0	73.4	LOSF	64.2	391.7	7 0.96	101 9	1.18	21.4

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MOVEMENT SUMMARY - 4.30pm - 5.30pm - Base Year 2024 - Orange Grove Road / Links Avenue / Golf Club Access

																				,							
Vehicle M	Aovement	: Performance	aou										3	hicle Mo	vement	Perform	auce										
Mov Tum 60	Turn Mov Class	Demand Flows (Total HV) vehill %	Authoral Flowers (Total HV)	Salah A	Aver Deslay	Level of Service		6% Back Of Queues Prop (Nets: Dect] velt: m	g d	野島豊	0 8 P	Speed Fruit	ž B		Tum Mev Class	Demand Total 17V oh h	Total H	In the second	Safe 3	Aver Les Jelay Sas sec	Service 95	95% Back Of Querue Prop. Qui (Voh. Diet) veh m	Guerue I Derf] m		Sign of the control o	No of State	And daily
South: Cum	H puerland H	Cumberland Hwy (670m)											So	South: Cumberland	H puelled	Hwy (670m)	_										
1 12			520.0				21.4				970	48.4	-		All MCs	520.0					986	50.9	150.0		95.0	6.63	49.1
2 TI		1642 2.9	1642 29					160.5	180		190	53.2	2	I	All MCs	1642 29	1642			10.2 LC	LOSA	21.4	153.3	0.63	85.0	0.63	25
3 R2	All MCs		0.0 8	0.768	11.4	LOSF	22.4			190	0.99	26.5	A A		All MCs	19 0.0	1666	3.0 0.0	0.768 1		LOSF	1.3	9.2		0.70	0.63	53.2
East Links Ave (360m)	Ave (360m												East	st Links A	Links Ave (360m)	30											
				0.356	63.0			17.3	0.97		0.97	13.9	4	77	All MCs	11 0.0	1				OSE	3.4	24.3		0.80	1 08	13.5
	All MCs			*		TOSE 1		17.3	0.97		0.97	18.5	LO.	F	All MCs	1 0.0	-				LOSF	3.4	24.3		080	1.05	18.1
6 R2		25 7.1	25 7.1				24	17.3	0.97	0.75	16.0	13.9	9	RZ	All MCs	36 5.0	38	5.0 0.	0.574 7	78.0 LC	LOSF	3.4	24.3	1.00	0.80	1.06	13.5
porce		35 5.1		0.356			24	173	0.97		160	14.1	App	eoude		48 3.8	48				JS F	3.4	24.3		0.80	1.05	13.7
North: Cumberland Hwy (240m)	therland the	my (240m)											No	North Cumb	erland H.	Cumberland Hwy (240m)											
7 12		22		0.112		LOSA	29	207			0.29		7		All MCs	62 0.0	62	0.0			LOSA	29	210		0.40	0.28	48.5
E 8	AI MCs	456 3.9	456 39				29	20.8	0.29	0.26	0.29	19.7	60	F	All MCs	456 339	456	3.9			LOSA	3.0	21.5	0.28	0.27	0.28	50.0
9 R2		8	8 0.0			1 108 F		4.1			0.99		0		All MCs	8 0.0	8	0.0			3S F	9.0	4.0		19:0	0.39	15.3
Approach		486 3.6	486 3.6	0.112		LOSA	29	308	030	0.27	0.30	46.4	Ap	Approach		526 3.4	526	3.4 0	0.120		LOSA	3.0	21.5	0.29	0.30	0.29	47.0
West Parking Access	ng Access	\$ (200m)											W	West: Parking Access	gAccess	(200m)											
10 12	All MCs	1911.1	19.11.1		61.6	3507 S		14.4	0.93	0.74	0.93	2.6	01		All MCs	19 11.1	19				LOSE	2.0	14.5		0.74	0.94	9.6
11			1 0.0	0.225			19	14.4	0.53		0.93	19.5	11		All MCs	1 0.0	-	0.0 0.0		67.2 10	LOSE	2.0	14.5		0.74	0.94	19.3
12 R2	All MCs	11 0.0	11 0.0			1 LOS F		14.4	0.93		0.93	5.6	12		All MCs	11 0.0	11				LOSF	2.0	14.5	0.94	0.74	0.94	9.6
Approach		31 6.9	31 6.9	0.225	65.1			14.4	0.93	0.74	0.93	10.2	Ap.	Approach		31 6.9	31	6.9			LOSE	2.0	14.5		0.74	0.94	10.0
All Vehicles	7720	2208 3.2	2208 32	0.768	121	LOSA	224	160.5	0.58	0.53	0.58	49.5	A	All Vehicles		2271 3.1	2271	3.1 0	0.768	12.0 LC	LOSA	21.4	153.3	950	25.0	950	49.3

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MOVEMENT SUMMARY - 4.30pm - 5.30pm - Base Year 2024 - Orange Grove Road / Viscount Place

		-	-e-	Dev	elop	Pre-Development	=				6/					5	5) >	2	Lost-Developinent					
Vehicle Movement Performance	ot Perform	92	_					10	_	4		Vehicle Movement Performance	vement	Perform	Jance							_			
Mov Turn Mov ID Class	Deniand Am Flows Flo Total HV [Total H vehith % vehith	Flower From From From From From From From Fro	S 8	Polar E	Aver Lovel of Delay Service REC	15% Flack	55% Back Of Queue Pmp. Que Veh. Dist veh. m	d d	S So FE	No of S Cycles	Speed	Mov Tum	Mov Special Pov Pov Pov Pov Pov Pov Pov Pov Pov Pov	Demand Amical Flows Flows [Total HV] [Total HV] velvh % velvh %	a a lateral and a second and a		Sell Sell Sell Sell Sell Sell Sell Sell	Aver Delay SF SF SF	Level of 95 Service	35% Back Of Queue Prop. Que [Veh. Dist] weh. m	(Queue F Dist.] m		Stop Stop	Aner No of Sp A	Speed
South: Orange Grove Road	ve Road											South: Orange Grove Road	де Grave	Road											
2 TI AIMOS	\$ 563 47	563 47	0.296			4.1	287	0.62	0.53	0.62	40.1	7 11	All MCs	574 4.6					LOSA	4.2					40.1
3 R2 AIMCs 580 26 580 26 Annmarch 1143 37 1143 37	1143 37	1143 37	*0.800	31.0	LOSC	7.9	999	100	0.97	1.30	36.7		All MCs	580 26		3.6	0.800	310 10	LOSC	7.9	9999	100	0.97	130	347
East Viscount Place	2	2				2	3				200	East Viscount Place	nt Place	5		3				3					
4 L2 AI MCs	3 741 17	741 17		14.4	LOSA	13.4	962	110	0.83	0.78	43.7	4 12	All MCs	741 17		17	0.707		LOSA	13.4					43.7
6 R2 AI MCs 766 19 766	8 766 19	766 19	+0.813			10.4	742	100	96.0	1.27	30.9	6 R2	All MCs	766	992 6	6		29.7 LC	DSC	10.4	742	1.00	96.0	127	608
Approach	1507 18 1507	1507 1.8	0.813	222	LOSB	13.4	962	980	16.0	1.03	37.2	peod		1507		1.8	0.813		LOSB	13.4					37.2
North: Orange Grove Road	e Road											North: Orange Grove Road	ge Grove	Road											
7 L2 AIMCs	\$ 447 10	447 1.0		12.5	LOSA	6.4	45.4	0.62	0.76	0.62	48.4		All MCs	447 1.0	0 447	1.0	0.436	12.5 LC	LOSA	6.4	45.4	0.62		0.62	49.4
8 T1 AI MCs	5 533 46	533 4.6	* *0.775	250	8501	7.1	518	100	96.0	1.26	41.5	E 8	All MCs	535 4.6	6 535	4.6	*0.778	25.1 LC	LOSB	7.2	52.2	1.00	96'0	1.26 4	41.5
Approach 979 3.0 979	979 3.0	979 3.0	0.775	19.3	LOSB	7.1	518	0.83	98.0	0.97	46.2	Approach		981 3.0		3.0	0.778	19.4 LC	HOS B	7.2	522	0.83	98.0	760	45.2
All Vehicles	3630 27	27 3630 27	0.813	20.6	BSOT	13.4	962	0.85	0.85	0.99	39.7	All Vehicles		3643 27	7 3643	27	0.813	20.6 LC	EOSB	13.4	952	0.85	0.85	66.0	39.7

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Appendix J – Proposed On-Street 'No Stopping' Signage & Line-Markings Plan

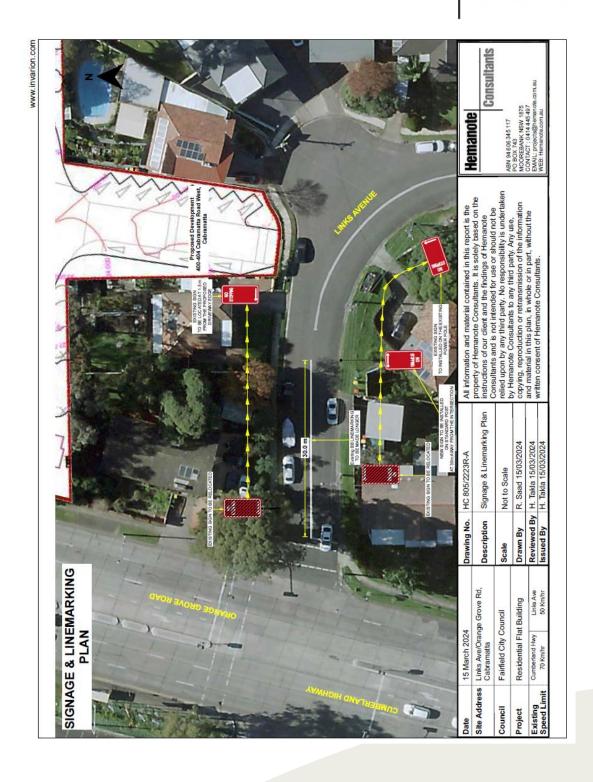
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Appendix K – Proposed On-Site Signage Plan (Traffic management measures)

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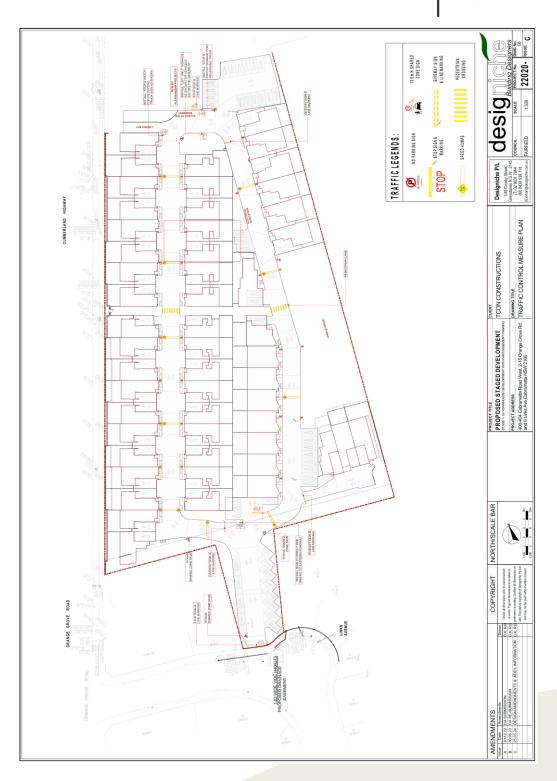
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